#### Advances in Water Resources 69 (2014) 79-94

Contents lists available at ScienceDirect

### Advances in Water Resources

journal homepage: www.elsevier.com/locate/advwatres

## Capture and release zones of permeable reactive barriers under the influence of advective–dispersive transport in the aquifer

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#### ARTICLE INFO

Article history: Received 20 May 2013 Received in revised form 27 March 2014 Accepted 28 March 2014 Available online 6 April 2014

Keywords: PRB Contaminant Plume remediation Capture efficiency Factor of safety Conformal mapping

#### ABSTRACT

The problem of permeable reactive barrier (PRB) capture and release behavior is investigated by means of an approximate analytical approach exploring the invariance of steady-state solutions of the advectiondispersion equation to conformal mapping. PRB configurations considered are doubly-symmetric funnel-and-gate as well as less frequent drain-and-gate systems. The effect of aquifer heterogeneity on contaminant plume spreading is hereby incorporated through an effective transverse macro-dispersion coefficient, which has to be known. Results are normalized and graphically represented in terms of a relative capture efficiency *M* of contaminant mass or groundwater passing a control plane (transect) at a sufficient distance up-stream of a PRB as to comply with underlying assumptions. Factors of safety *FS* are given as the ratios of required capture width under advective–dispersive and purely advective transport for achieving equal capture efficiency *M*. It is found that *M* also applies to the release behavior down-stream of a PRB, i.e., it describes the spreading and dilution of PRB treated groundwater possibly containing incompletely remediated contamination and/or remediation reaction products. Hypothetical examples are given to demonstrate results.

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#### 1. Introduction

Permeable reactive barriers (PRBs) are a popular technique for passive long-term interception and treatment of contaminant plumes in aquifers due to their cost effectiveness compared to pump and treat systems [3,28,40]. PRBs basically consist of a type of reactive material, which is installed in the pathway of a contaminant plume (e.g., in a trench across ambient groundwater flow driven by a natural gradient) and which degrades or retains contamination through chemical, biological or physical processes during the contaminant residence (or travel) time inside the reactive material [8,22,23,30]. For an effective application it is important that the PRB captures the target portion of the contaminated groundwater plume and that the contaminant residence time within the reactive material is adequate to achieve treatment objectives. In order to meet both requirements under a variety of conditions, different PRB configurations have been applied or

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proposed including (a) funnel-and-gate (FG) PRBs with impermeable funnel arms to increase the width of the capture zone; (b) velocity equalization walls (VEW) to achieve more uniform contaminant residence times in a reactor; and (c) drain-and-gate (DG) PRBs using trench-like drains to capture and release groundwater before and after passing a reactor. While FG PRBs, which may be reduced to continuous wall PRBs by using zero funnel length, are predominant, DG PRBs are limited to a smaller number of field applications (e.g., [7,37,42]). To the best of our knowledge, VEW PRBs have so far only been proposed conceptually [39]. Examples of these PRB types with groundwater stream lines, potential lines and capture zones are shown in Fig. 1.

Theoretical studies of aquifer hydraulics are complicated by the hydraulic conductivity contrast between aquifer and reactive materials as well as by the presence of impermeable (e.g., funnel) or highly permeable (e.g., drains) PRB structure elements. As a consequence, analytical solutions (e.g., [2,15,31–33]) are less frequent and numerical approaches have been applied predominantly (e.g., [29,36,39,43–45]). The hydrodynamic conditions are further complicated by the natural heterogeneity of the aquifer, most notably by the spatial variability of hydraulic conductivity. This spatial





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#### Nomenclature

Dimensio	Dimensionless		transverse distance from a point source in uniform flow
В	relative capture width for $R = 0$	$\Delta x$	distance between transect and PRB
E()	incomplete (with two arguments) or complete (with	$\Delta \varphi$	head drop required to drive flow Q through reactor
-()	one argument) elliptic integral of the second kind	$\phi^{'}$	potential function normalized to unit $q_0$
F()	incomplete elliptic integral of the first kind	$\Phi_0$	potential function at line source (transect) location
	standard normal cumulative distribution function (cdf)	$\Phi_{\rm P}$	potential function at capture point P
F <sub>N</sub> () FS			potential function at point source location
гs	factor of safety (relative increase in capture width re-	$\Phi_{point} \ \Phi^{*(*)}$	normalized potential function defined by Eqs. (11) and
	quired to achieve equal M under advective-dispersive	$\Psi^{(i)}$	
	as under purely advective transport)		(14)
I()	normalized indefinite integral defined by Eq. (10)	Ψ	stream function normalized to unit $q_0$
K()	complete elliptic integral of the first kind	$\Psi_{peak}$	stream function at peak of Gaussian source distribution
$K_0()$	modified Bessel function of the second kind and order	$\Psi_{point}$	stream function at point source location
	zero	$\Psi_P$	half of capture width in stream line coordinates
Μ	capture efficiency (portion of contaminant mass dis-	$\Psi_{P1,P2}$	stream function at capture points
	charge captured by PRB)	$\Psi_0$	half of transect length in steam line coordinates
R	hydraulic reactor resistance defined by Eq. (21)	$\Psi_{01.02}$	stream function at extremes of line source (transect)
R <sub>max</sub>	maximum value of <i>R</i> to avoid flow divergence around	$\Psi^{*(*)}$	normalized stream function defined by Eqs. (12) and
IIIax	reactor		(15)
$e_{(w)}$	auxiliary variables defined in Appendix C	$\Omega$	complex potential normalized to unit $q_0$
$f_N()$	standard normal probability distribution function (pdf)	$\alpha_l$	longitudinal dispersivity
i	imaginary unit	$\alpha_t$	transverse dispersivity
$k^{(1)}$	(complementary) modulus of elliptic integral	$\varphi$	hydraulic head
	aquifer porosity	,	standard deviation of Gaussian source distribution
n A .:		$\sigma_0$	standard deviation of Gaussian source distribution
$\Delta \alpha_t$	change (uncertainty) in $\alpha_t$		
$\Delta FS$	change (uncertainty) in FS	Others	
$ au_1$	auxiliary conformal mapping plane	С	contaminant concentration in the aqueous phase [M/L <sup>3</sup> ]
		$C_{\text{point}}$	contaminant concentration corresponding to a point
Lengths			source in uniform flow under effects of transverse dis-
Н	average saturated thickness of flow in the reactor		persion [M/L <sup>3</sup> ]
L	travel distance from a contaminant source	$C_{tl}$	contaminant concentration corresponding to a point
а	reactor length (perpendicular to design flow direction)		source in uniform flow under effects of transverse and
b	reactor width (along design flow direction)		longitudinal dispersion [M/L <sup>3</sup> ]
С	scaling factor for conformal mapping	$D_l$	effective longitudinal macro-dispersion coefficient [L <sup>2</sup> /
f	length of funnel arms		T]
S	curvilinear length coordinate along stream lines	$D_t$	effective transverse macro-dispersion coefficient $[L^2/T]$
t	curvilinear length coordinates transverse to stream	κ <sub>f</sub>	hydraulic aquifer conductivity [L/T]
L	lines	Q	flow through reactor $[L^3/T]$
w	length of velocity equalization walls (VEW)	m	constant describing mass release rate of contaminant
x	Cartesian coordinate		point source $[M/L^2]$
	x-coordinate of transect	<i>a</i> <sub>c</sub>	specific discharge of undisturbed ambient groundwater
$x_0$	Cartesian coordinate	$q_0$	flow [L/T]
y v		11	magnitude of local pore water velocity [L/T]
$y_0$	half of transect length in y-direction	$v_s$	magintude of local pole water velocity [L/1]
$\Delta s$	longitudinal distance from a point source in uniform		
	flow		

variability is never fully characterized in the field giving rise to stochastic approaches, which honor some basic (geo) statistical parameters of hydraulic conductivity (e.g., mean, variance, correlation scale) and possibly some conductivity measurements at a limited number of locations. These stochastic approaches have so far been constrained to numerical solution methods of the aquifer hydraulics and include (to different levels of complexity) the work of Gupta and Fox [26], Bilbrey and Shafer [5], Elder et al. [19], Cirpka et al. [13], Hemsi and Shackelford [27] and Bürger et al. [9].

The present work focuses on an approximate analytical solution to the PRB capture and release behavior in the presence of aquifer heterogeneity and advective–dispersive transport due to a natural ambient gradient. Modeling of the remediation reactions in a PRB reactor is not considered here. As thoroughly discussed in the literature on stochastic groundwater hydrology (e.g., [16,24,41]), aquifer heterogeneity at a local (i.e., 1–10 m) scale leads to a well-known phenomenon called macro-dispersion (as opposed to micro-dispersion at pore scale). This results in a significant longitudinal and transverse spreading of contaminant plumes with travel distance or, equivalently, in a mixing of groundwater in the longitudinal and transverse directions to flow. Although more general in principle, our approach takes advantage of the conformal mapping solutions of Klammler and Hatfield [32] and Klammler et al. [33] to map a simple solution of the steady-state advection-dispersion equation onto flow domains containing the PRB configurations of Fig. 1. An effective macro-dispersion coefficient may be chosen to account for aquifer heterogeneity and a possibly variable ambient groundwater flow direction. By the assumption of Fickian transport, results become approximate for finite travel distances and provide some theoretical insight about (1) the portion of groundwater or contaminant mass (under simplified plume scenarios) passing an up-stream control plane (transect) and being captured by a PRB, as well as (2) the down-stream spreading and dilution of treated groundwater released by a PRB. The following sections

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