



Fatigue performance evaluation of modified asphalt binder using of dissipated energy approach



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HIGHLIGHTS

- The modified asphalt binders perform better at high strain levels.
- The initial dissipated energy has the potential to become a rapid method to describe the fatigue resistance of asphalt.
- In nonlinear strains, the curve slope of modified binder is less than curve slope of pure binder.
- In nonlinear strains, unmodified and modified asphalt binders with increasing loading cycles, DE decreased.

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ABSTRACT

In this paper dissipated energy (DE) versus loading cycles is plotted, at the same time fatigue life of asphalt binders were investigated using of dissipated energy (DE) approach and based on linear and non-linear strain levels. It was found that based on results obtained of time sweep test; there is a fair correlation between dissipated energy at the 50th loading cycle and fatigue life of asphalt binders in all strain levels. As a result, initial dissipated energy (IDE) is a parameter obtained from time sweep test, so the amount of initial dissipated energy (IDE) at the 50th loading cycle can be predicted the fatigue of asphalt binders without performing millions of loading cycles.

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1. Introduction

Bituminous materials are widely used in construction of flexible pavements. Fatigue cracking is one of the major distresses of flexible pavements and is defined as the damage in asphalt pavements by repetitive stresses and strains due to traffic loading and environmental factors [1,2,3]. According to report NCHRP-RPT-459, there are several factors effective in estimating and prediction

fatigue life asphalt binders [4,5]. The six most important factors that influence on fatigue life asphalt binders are binder type, strain level, temperature, frequency, number of cycles and rest period. It has been shown that the asphalt mixture fatigue resistance is strongly correlated with asphalt binder fatigue properties at intermediate temperatures [6,7,9,9].

Utilizing the concept of energy in assessing the fatigue behavior of asphalt binders has two advantages. First, the energy is a scalar function which is independent of the direction of the applied stress or strain. Secondly, it includes stress, strain the material's properties in a single unit of measure [22]. Evaluation of binder fatigue performance under cyclic loading has continuously been one of the most popular research topics for engineers during the past few years [10].

Airey et al. [8,11] conducted stress sweep tests using of DSR to obtain the linearity limits of various asphalt binders at different

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temperatures. Furthermore, Bahia in 1999 reported the fatigue performance of asphalt binders at various strain levels in the linear and nonlinear range and their effects on mixture performance [4,5].

There are several developments in terms of asphalt binder fatigue tests which time sweep test, according to project NCHRP 9-10, has reported to be an effective test method to evaluate binder fatigue life [4,6,7,12]. So that Time sweep (TS) test is reported to be a promising method to estimate fatigue performance of asphalt binders. This test provides a simple method of applying repeated cycling of stress or strain loading at selected temperatures and loading frequencies. Another advantage of this test is that it can be used to calculate fatigue life of asphalt binders based on dissipated energy approach. Dissipated energy approach has been broadly used as a simplified method to characterize fatigue performance of viscoelastic materials based on modulus and phase angle in each loading cycles. Van Dijk introduced the approach to evaluate fatigue performance of viscoelastic materials in terms of dissipated energy [13,14]. He proposed equation (1) to calculate dissipated energy during a single load cycle.

$$W_n = \pi \sigma_n \varepsilon_n \sin \theta_n \quad (1)$$

where W_n is the dissipated energy, σ_n and ε_n are the applied stress and strain, θ_n is the phase angle and n represents number of loading cycles. In this research study in order to have a more meaningful of the fatigue properties of asphalt binders, the dissipated energy

approach was utilized. According to this concept, the dissipated energy in each cycle can be determined by Eq. (1).

Cheng et al. (2002) showed that the energy balance is influenced by rheological properties of the mix and the asphalt binder, and also fatigue damage in viscoelastic materials can be due to stored and dissipated energies [15]. Eq. (2) was proposed to characterize the number of cycles to failure using dissipated energy. Where: N_f is number of load application to failure, W_i is dissipated energy and K_1 , K_2 are experimentally determined coefficients.

$$N_f = K_1 \left(\frac{1}{W_i} \right)^{K_2} \quad (2)$$

Baburamani and Porter [16], Rowe [17] and Ghuzlan [18] indicated that initial dissipated energy also can be an accelerate method to evaluate fatigue life of viscoelastic materials [16,17,18]. Initial dissipated energy (IDE) is defined as the dissipated energy measured at 50th loading cycle. However, Carpenter and Shen (2003) found that the initial dissipated energy approach is not appropriate for the whole loading range, especially at low strain fatigue tests [19].

The objective of this study is to determine the linear and nonlinear strain ranges of six asphalt binders in a constant frequency using of strain sweep test. In during to strain sweep tests, for determining of linear and nonlinear range, strain 0–20% applied on all asphalt binder samples. Next, dissipated energy versus loading cycles is plotted on linear and non linear strain levels. According

Table 1
The properties of neat binders used in this study.

Parameters	Unit	Test methods		60/70	85/100 Tabriz
		ASTM	AASHTO		
Specific gravity	g/cm ³	D70	T228	1.016	1.009
Penetration (100 g)	0.1 mm	D5	T49	66	99
Softening point	°C	D36	T53	51	46.4
Ductility	Cm	D113	T51	>100	>100
Solubility	(%)	D2042	T44	99.5	99.56
Flash point (Cleveland)	°C	D2170	T201	303	260
Kinematic viscosity 135 °C	Centistokes	D6	–	361	240
Heating loss	(%)	–	–	0.04	0.15
Penetration after heating loss	0.1 mm	–	–	47	66
Penetration after heating loss original penetration	(%)	–	–	85.4	66
Ductility after heating loss	cm	–	–	>50 cm	80
Penetration Index (PI)	–	–	–	–0.73	–0.3
PVN(25-135)	–	–	–	–0.39	3.21

Table 2
Properties of SBS and CR used in this study.

Physical properties	Unit	Specification	Test method		
Density	g/cm ³	0.94	ISO 2781		
Volatile material	wt%	Max. 0.5	ASTM D1416		
Styrene content	%	31	LSYQS1D011		
Mechanical Properties					
Melt index	g/10 min	<1	ASTM D1238		
Hardness T	Shore A	79	ASTM D2240		
T.S.V.	cst	13.4	ASTM D445		
Other					
Appearance	–	Pellet	–		
Structure Melt Index: 200 C, 5 kg	–	Linear	–		
Crumb rubber			ACR	CCR	
	Specific gravity (gcm ³)		1.042		1.053
	Moisture content (wt%)		0.76		0.77
	Ash content (wt%)		6.01		4.66
	Carbon black content (wt%)		32.98		30.41
	Extract content (acetone and chloroform) (wt%)		9.86		11.69
	Sulphur content * (wt%)		2.02		1.24
	Source		Car tires		Car tires

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