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Transportation Research Procedia 7 (2015) 615 - 630



21st International Symposium on Transportation and Traffic Theory

# Stochastic Approximations for the Macroscopic Fundamental Diagram of Urban Networks

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#### **Abstract**

This paper proposes a theory for estimating the Macroscopic Fundamental Diagram (MFD) on inhomogeneous corridors and networks using probabilistic methods. By exploiting a symmetry property of the stochastic MFD, whereby it exhibits identical probability distributions in free-flow and congestion, it is found that the network MFD depends mainly on two dimensionless parameters: the mean block length to green ratio and the mean red to green ratio. The theory is validated with an exact traffic simulation and with the empirical data from the city of Yokohama. It is also shown that the effect of buses can be approximated with the proposed theory by accounting for their effect in the red to green ratio parameter.

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Selection and peer-review under responsibility of Kobe University

Keywords: Microscopic Fundamental Diagram; Traffic flow; Urban congestion

#### 1. Introduction

It has been shown experimentally by Geroliminis and Daganzo (2008) that the average flow on an urban network can be accurately predicted knowing the average density in the network. This urban-scale Macroscopic Fundamental Diagram (MFD) appears as an invaluable tool to overcome the difficulties of traditional planning models. Although it is still under debate whether it depends on trip origins and destinations and route choice, there is no question that network topology and control parameters such as block length, existence of turn-only lanes, and traffic light settings play a key role.

Existing methods to estimate the MFD analytically for simple homogeneous arterial corridors can be categorized into three types: (i) empirical (Geroliminis and Daganzo, 2007, 2008; Wu et al., 2011; Saberi and Mahmassani, 2012; Geroliminis and Sun, 2011; Geroliminis and Ji, 2011; Cassidy et al., 2011; Knoop, 2012; Gayah and Daganzo, 2011; Buisson and Ladier, 2009; Daganzo et al., 2011), (ii) analytical (Daganzo and Geroliminis, 2008; Leclercq et al., 2014), and (iii) simulation (Ji et al., 2010; Mazlomian et al., 2010; Geroliminis and Boyacı, 2013; Haddad and Geroliminis, 2012; Haddad et al., 2013; Knoop and Hoogendoorn, 2011; Knoop et al., 2011). Existing analytical results are based on the method of cuts for homogeneous corridors, i.e. with equal block size, signal settings and constant offset, and therefore, one can focus on the cuts from a single intersection to compute the MFD for the whole corridor. Despite this apparent simplicity, this approach quickly becomes intractable, more so if buses are

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introduced (Chiabaut et al., 2014). Even with no buses the homogeneous corridor method cannot be scaled up without complications to estimate the network MFD mainly because a network path cannot be guaranteed to have constant offset all along, even for homogeneous networks.

To overcome these difficulties, in this paper we introduce the concept of stochastic corridors, where any particular inhomogeneous corridor—with different block lengths and signal timing—is seen as a particular realization. Stochastic corridors are in fact probabilistically homogeneous in the sense that the distribution of these network parameters does not change in time or space. This approach allows the estimation of the network MFD, for which analytical methods are currently unavailable.

This paper is organized as follows. Section 2 develops the theory of stochastic corridors, which is based on renewal theory. The existence of short blocks is examined in detail in section 3, as it can severely reduce corridor capacity. Section 4 is devoted to comparing the theory both with an exact traffic simulation and the empirical data from the city of Yokohama presented in Geroliminis and Daganzo (2008). Finally, section 5 presents a discussion.

#### 2. Stochastic corridors

Consider an inhomogeneous corridor consisting of a large sequence of road segments of different length, each one delimited by a traffic signal with settings that vary in time and across segments. This particular corridor is viewed here as a realization of a "stochastic corridor" random variable, where the length of each segment and the red and green times of its signals are random variables  $\ell$ , r and g, respectively, assumed to be independent. We use the symbols  $\mu$ ,  $\sigma$ and  $\delta = \sigma/\mu$  for the mean, standard deviation, and coefficient of variation of a random variable, whose name will be indicated as subscript; e.g., block lengths are assumed i.i.d. with mean and variance  $\mu_{\ell}, \sigma_{\ell}^2$ . Turning movements are not considered in our analysis.

We use the superscripts "#" and "b" to differentiate variables pertaining to forward and backward cuts, respectively, while the superscript "-" will be used as their placeholder. All links in the network are assumed to obey a triangular fundamental diagram (FD) with free-flow speed  $w^{\sharp}$ , wave speed  $-w^{\flat}$  and jam density  $\kappa$ ; the saturation flow is therefore  $Q = \kappa w^{\flat} w^{\sharp} / (w^{\flat} + w^{\sharp}).$ 

Our formulation is based on variational theory Daganzo (2005a,b), which corresponds to the solution of the kinematic wave model of Lighthill and Whitham (1955); Richards (1956) when expressed as a Hamilton-Jacobi partial differential equation. This solution-known as the Hopf-Lax formula (Lax, 1957; Hopf, 1970)-states that the number of vehicles that have crossed location x by time t, N(t, x), can be expressed in variational form as:

$$N_P = \inf_{B \in \mathcal{B}_n} \{ N_B + \Delta_{BP} \} \tag{1}$$

where P is a generic point with coordinates (t, x),  $\mathcal{B}_P$  is the set of all points in the boundary that are in the domain of dependence of P, the point  $B \equiv (t_B, x_B)$  is in  $\mathcal{B}_P$ ,  $N_P \equiv N(t, x)$  and  $N_B \equiv N(t_B, x_B)$ , and  $\Delta_{BP}$  is the "cost" or maximum number of vehicles that can cross the minimum path joining B and P; see Fig. 1a. (Notice that in the absence of bottlenecks, such as traffic lights, all valid paths-including the minimum path-between B and P have the same cost and it is customary to compute  $\Delta_{BP}$  along the straight line BP.)

To derive the corridor MFD, consider the initial value problem in Fig. 1b where the vehicle number  $N(t_B, x_B)$  is known in the boundary  $t_B = 0$  such that the density, k, is constant. Noting that in this case,  $N_B = N_O + (x - x_B)k$  with  $N_O \equiv N(0, x)$ , we can write

$$N_P - N_O = \min_{R} \{ \Delta_{BP} + (x - x_B)k \}. \tag{2}$$

The MFD is defined as the steady-state flow at any location x; i.e.:

$$q(k) \equiv \lim_{t \to \infty} \frac{1}{t} (N_P - N_O) \tag{3a}$$

$$= \min_{B} \{ \lim_{t \to \infty} \frac{1}{t} (\Delta_{BP} + (x - x_B)k) \},$$

$$= \min_{V} \{ \phi(V) + Vk \}.$$
(3b)

$$= \min_{v} \{\phi(v) + vk\}. \tag{3c}$$

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