



# Variational formulation and finite element analysis for nonlocal elastic nanobeams and nanoplates

J.K. Phadikar, S.C. Pradhan\*

Department of Aerospace Engineering, Indian Institute of Technology, Kharagpur, West Bengal 721 302, India

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## ABSTRACT

In the present work, finite element formulations for nonlocal elastic (i) Euler–Bernoulli beam and (ii) Kirchhoff plate have been reported. Nonlocal differential elasticity theory is considered. Galerkin finite element technique has been employed. For both nanobeams and nanoplates weak forms of governing equations are derived and energy functionals are obtained. Present finite element results for bending, vibration and buckling for nonlocal beam with four classical boundary conditions are computed. These results are in good agreement with those reported in the literature. Further, bending, vibration and buckling analyses are carried out for stepped nanobeam. Furthermore, using present finite element bending, vibration and buckling analyses for nonlocal nanoplate are carried out. Present formulation will be useful for structural analyses of nanostructures with complex geometry, material property, loading and boundary conditions.

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## 1. Introduction

Due to outstanding physical, chemical, mechanical and electronic properties [1–4], nano-sized structures [5–8] have attracted a great deal of attention in scientific community. As conduction of experiments in nano-level are difficult to control and theoretical atomistic models are computationally intensive for relatively large scale nanostructures, the continuum and semi-continuum [9–11] models have been proven to be important tools in the study of the nanostructures. Considering the continuum models, the use of traditional elasticity theory may lead to erroneous results as continuum assumption may not hold valid in the small scales. This fact triggered development of various micro-continuum theories such as couple stress theory [12], micro-morphic theory [13], strain gradient elasticity theory [14] and nonlocal elasticity theory [15]. Among these theories, nonlocal elasticity theory has been widely applied to various problems of physics including lattice dispersion of elastic waves and dislocation mechanics [16].

Chen et al. [17] proved that nonlocal elasticity theory is consistent with the molecular dynamics. This has made the nonlocal elasticity theory an efficient alternate to atomistic methods. Initially Peddieson et al. [18] applied present nonlocal elasticity theory and studied flexural behavior of one dimensional nanostructures. Since then a large number of research activities using nonlocal elasticity theory have taken place. These include analyses of nanobeams [19–26], nanoplates [27–32] and nano-shells [33–35]. In the analyses of nanostructures, nonlocal differen-

tial elasticity (or nonlocal stress gradient elasticity) has gained more popularity among researchers as compared to the nonlocal integral elasticity [33] due to its simplicity.

So far two methodologies have been used extensively for solving the governing differential equations arising in structural analysis of nonlocal elastic nanostructures. These are Navier's method [27,28,32] and Differential Quadrature Method (DQM) [20,21,29,30]. However it is well known that finite element method in contrast to these methods can effectively handle more complex geometry, material property, boundary and/or loading conditions. Construction of variational principles in the framework of nonlocal integral elasticity has been reported by Polizzotto [36]. Pisano et al. [37,38] reported the finite element procedure for nonlocal integral elasticity. Recently Adali [39,40] and Shakouri et al. [41] have devised traditional variational methods based on nonlocal differential elasticity for the analysis of carbon nanotube. However to the authors' best knowledge, the finite element analysis of nonlocal elastic structures with nonlocal differential elasticity approach is not available in the open literature. In the present work, finite element formulation for nonlocal differential elasticity approach of Euler–Bernoulli beam theory and classical plate theory have been reported. Expressions for quadratic functionals have also been given.

## 2. Review of governing equations

### 2.1. Nonlocal beam

Formulation for bending, vibration and buckling of nonlocal Euler–Bernoulli beam has been reported by Reddy [42]. To write the

\* Corresponding author. Tel.: +91 3222 283008; fax: +91 3222 282242.

E-mail address: [scp@aero.iitkgp.ernet.in](mailto:scp@aero.iitkgp.ernet.in) (S.C. Pradhan).

**Nomenclature**

$a, b$	length and width of the plate element	$Q_x, Q_y$	shear stress resultants for the plate
$[B^b], [B^p]$	buckling stiffness matrix for beam and plate respectively	SS, CL, FR	simply-supported, clamped and free boundary conditions respectively
$D_{11}, D_{12}, D_{22}, D_{66}$	bending rigidities of the plate	$u_i^b, u_i^p$	degree of freedom for beam and plate respectively
$Eb$	Young's modulus of the beam material	$[U^b], [U^p]$	degree of freedom vector for beam and plate respectively
$E1, E2$	Young's moduli of the plate material	$Vb$	shear stress resultant for the beam
$Fb, Fp$	functional for beam and plate respectively	$\hat{w}^b, \hat{w}^p$	non-dimensional maximum deflection of beam and plate respectively
$G12$	shear modulus of plate the plate material	$w^b$ (or $W^b$ ), $w^p$ (or $W^p$ )	deflection of beam and plate respectively
$h$	thickness of the plate	$\chi^b, \chi^p$	weight function for beam and plate respectively
$[K^b], [K^p]$	stiffness matrix for beam and plate respectively	$\epsilon_{xx}$	axial strain tensor for the beam
$l$	length of the beam element	$\lambda^b, \lambda^p$	ratio of actual buckling load and applied in-plane loads for beam and plate respectively
$m_0^b, m_0^p$	mass moment of inertia for beam and plate respectively	$\mu$	nonlocal parameter
$Mb$	moment resultant for the beam	$\nu_{12}, \nu_{21}$	Poisson's ratios of plate
$M_1^{xx}, M_1^{yy}, M_1^{xy}$	moment resultants for the plate	$\rho^b, \rho^p$	density of the beam material and plate material respectively
$(n, s)$	local tangential and normal coordinate at the boundary	$\omega^b, \omega^p$	frequency of beam and plate respectively
$N_i^b, N_i^p$	interpolation function for beam and plate respectively	$\hat{\omega}^b, \hat{\omega}^p$	non-dimensional natural frequency of beam and plate respectively
$N_0^{xx}, N_0^{yy}, N_0^{xy}$	in-plane stress resultants for plate	$\sigma_{xx}$	axial stress tensor for the beam
$[M^b], [M^p]$	mass matrix for beam and plate respectively	$\nabla^2$	Laplacian operator in two dimensional cartesian coordinate system
$M_n, M_{ns}, V_n$	boundary stress resultants		
$P$	axial load for the beam		
$\hat{P}_b^b, \hat{P}_{cr}^p$	critical buckling loads for beam and plate respectively		
$q^b, q^p$	transverse distributed load for beam and plate respectively		

governing equations, we assume the follow reference coordinate system:  $x, y$  and  $z$  axes are taken along the length, width and thickness of the beam, respectively. Using the principle of virtual work following equilibrium equations are obtained:

$$\frac{\partial^2 M^b}{\partial x^2} + q^b - P \frac{\partial^2 w^b}{\partial x^2} - m_0^b \frac{\partial^2 w^b}{\partial t^2} = 0 \tag{1}$$

$$V^b - \frac{\partial M^b}{\partial x} + P \frac{\partial w^b}{\partial x} = 0 \tag{2}$$

The constitutive relation according to nonlocal differential elasticity takes the following form for the beam:

$$\sigma_{xx} - \mu \frac{\partial^2 \sigma_{xx}}{\partial x^2} = E^b \epsilon_{xx} \tag{3}$$

The stress resultants can be written in terms of displacements as follows:

$$M^b = -(E^b I^b - \mu P) \frac{\partial^2 w^b}{\partial x^2} - \mu q^b + \mu m_0^b \frac{\partial^2 w^b}{\partial t^2} \tag{4}$$

$$V^b = -(E^b I^b - \mu P) \frac{\partial^3 w^b}{\partial x^3} - \mu \frac{\partial q^b}{\partial x} + \mu m_0^b \frac{\partial^3 w^b}{\partial x \partial t^2} - P \frac{\partial w^b}{\partial x} \tag{5}$$

Putting Eq. (4) into Eq. (1) and assuming,  $w^b(x, t) = W^b(x)e^{i\omega t}$  we get the following governing equation in terms of displacement:

$$E^b I^b \frac{\partial^4 W^b}{\partial x^4} - \left(1 - \mu \frac{\partial^2}{\partial x^2}\right) \left\{ q^b(x) - P \frac{\partial^2 W^b}{\partial x^2} + m_0^b \omega^2 W^b - m_0^b \omega^2 \frac{\partial^2 W^b}{\partial x^2} \right\} = 0 \tag{6}$$

It can be noted that, by putting  $\mu = 0$ , in Eq. (6), governing equation for a local beam can be retrieved.

**2.2. Nonlocal plate**

Formulation for bending, vibration and buckling of nonlocal plate based on classical plate theory has been reported by Pradhan and Phadikar [27,28,46]. Following coordinate system has been

chosen to write the equations. Origin is chosen at one corner of the plate.  $x, y$  and  $z$  coordinate axes are taken along the length, width and thickness of the plate, respectively. Following equilibrium equations is obtained using the principle of virtual work:

$$\frac{\partial^2 M_1^{xx}}{\partial x^2} + 2 \frac{\partial^2 M_1^{xy}}{\partial x \partial y} + \frac{\partial^2 M_1^{yy}}{\partial y^2} + q^p + \frac{\partial}{\partial x} \left( N_0^{xx} \frac{\partial w^p}{\partial x} \right) + \frac{\partial}{\partial y} \left( N_0^{yy} \frac{\partial w^p}{\partial y} \right) + \frac{\partial}{\partial x} \left( N_0^{xy} \frac{\partial w^p}{\partial y} \right) + \frac{\partial}{\partial y} \left( N_0^{xy} \frac{\partial w^p}{\partial x} \right) - m_0^p \frac{\partial^2 w^p}{\partial t^2} = 0 \tag{7}$$

$$Q_x - \frac{\partial M_1^{xx}}{\partial x} - \frac{\partial M_1^{xy}}{\partial y} - N_0^{xx} \frac{\partial w^p}{\partial x} - N_0^{xy} \frac{\partial w^p}{\partial y} = 0 \tag{8}$$

$$Q_y - \frac{\partial M_1^{xy}}{\partial x} - \frac{\partial M_1^{yy}}{\partial y} - N_0^{xy} \frac{\partial w^p}{\partial y} - N_0^{yy} \frac{\partial w^p}{\partial x} = 0 \tag{9}$$

Using the nonlocal constitutive relation, stress resultants are written in terms of displacements as follows:

$$M_1^{xx} - \mu \nabla^2 M_1^{xx} = -D_{11} \frac{\partial^2 w^p}{\partial x^2} - D_{12} \frac{\partial^2 w^p}{\partial y^2} \tag{10}$$

$$M_1^{yy} - \mu \nabla^2 M_1^{yy} = -D_{12} \frac{\partial^2 w^p}{\partial x^2} - D_{22} \frac{\partial^2 w^p}{\partial y^2} \tag{11}$$

$$M_1^{xy} - \mu \nabla^2 M_1^{xy} = -2D_{66} \frac{\partial^2 w^p}{\partial x \partial y} \tag{12}$$

Here  $D_{ij}$  are various bending rigidities defined as

$$D_{11} = \frac{E_1 h^3}{12(1 - \nu_{12}\nu_{21})}, \quad D_{12} = \frac{\nu_{12} E_2 h^3}{12(1 - \nu_{12}\nu_{21})},$$

$$D_{22} = \frac{E_2 h^3}{12(1 - \nu_{12}\nu_{21})}, \quad D_{66} = \frac{G_{12} h^3}{12} \tag{13}$$

Using Eqs. (10)–(12) and Eq. (7) and assuming  $w^p(x, t) = W^p(x)e^{i\omega t}$  we get the following governing equation in terms of displacements:

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