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## Non-linear dynamic response to simple harmonic excitation of a thin-walled beam with a breathing crack



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#### ABSTRACT

The dynamic response of a thin-walled beam with a breathing crack is studied by employing a refined one-dimensional model introduced for such purpose. It is shown that, due to the non-linearity of breathing, some effects take place which are impossible by using a completely open crack model. Even with the simplest of sinusoidal excitations, the system under study reveals a rich and complex dynamics. Some of the topics emphasized in the article are self-excitation of harmonic resonances, period doubling and the presence of quasi-periodic motion. Furthermore, the possibility of chaotic vibrations is analyzed. © 2015 Elsevier Inc. All rights reserved.

#### 1. Introduction

The presence of damage represents a crucial topic regarding safety of engineering structures. Since early damage detection may prevent the possibility of service failures, the development of structural health monitoring techniques has acquired a remarkable importance in recent years [1–3]. A very common damage typology that appears in dynamic operating condition corresponds to fatigue cracks. These cracks are usually invisible to the naked eye and their identification by local non-destructive methods, such as dye penetrant and ultrasonic testing, may be impractical if the structure must remain in service.

These aspects lead to the need of mathematical models as interpretative tools for crack identification, which is generally performed by solving an inverse optimization problem [4]. This problem is usually complex from a mathematical point of view, and also must be solved quickly. Hence models must combine precision and simplicity in balanced form. For slender structures, a beam model captures the most significant features of the structural dynamics. And at the same time it is simple enough for extensive computational treatment, an essentially useful feature when damage diagnosis must be performed in real time. Most studies have focused on Bernoulli–Euler beams [5–9] and, to a lesser extent, on Timoshenko beams [10–13]. Thin-walled beams have not been addressed in depth, perhaps due to their more complex dynamics, usually presenting a strongly coupled vibrational response [14–22].

Crack models are generally assumed to be linear, with a crack remaining open during vibration. But while this hypothesis works well for many situations, it is more realistic to consider a fatigue crack which may be open or closed during the motion [10,23,24]. This "breathing effect" introduces a non-linearity in the system since the stiffness of the structure becomes time

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Nomenclature	
A	crack denth (also semi-major axis of the elliptic crack)
ã	variable crack depth
A	cross-sectional area
$A_f$	loading amplitude
b	dimension of a flange
В	bimomental beam force (also point <i>B</i> , origin of the system <i>B</i> : <i>x</i> , <i>s</i> , <i>n</i> )
С	semi-minor axis of the elliptic crack
C	center of gravity of the uncracked cross section
C <sub>w</sub>	damping matrix
F	Volue's modulus
f	driving frequency, expressed in Hz
f <sub>B</sub>	bilinear frequency, expressed in Hz
$\mathbf{F}_{C}^{(e)}$	flexibility matrix of the cracked element
Ğ	shear modulus
<i>G</i> *	crack mouth widening energy release rate
h	dimension of the web
H	point placed at the cracked branch of the cross-section
I <sub>y</sub> , I <sub>Z</sub> I	product moment of area
$I_{yz}$	product of warping
Iyw, 1200 Isv	Saint-Venant's torsion constant
I <sub>ii</sub>	shear coefficients of the constitutive matrix
Ĵ	Rice's J integral
J	constitutive matrix of the beam
K	point placed at the cracked branch of the cross-section
<b>К</b> 0 И	global stiffness matrix of a hear with a fully open study
K <sub>C</sub> V	global stiffness matrix of a beam with a breathing crack
$\mathbf{K}^{(e)}_{B}$	stiffness matrix of the cracked element
L.	length of the beam
$\overline{M}_{x}$	total torque
М	inertia matrix of the uncracked beam
п	coordinate normal to the cross-section middle line
N	axial beam force
$M_{y}, M_{z}$	bending moments
$\mathbf{p}^{(i)}$	vector of nodal forces of node <i>i</i>
$P_i$	nodal forces
Q r	distance in s-direction from C to a point in the cross-section middle line
r S	circumferential coordinate
S	cross-sectional perimeter
$S_{v}, S_{z}$	first moments of area
Śω	first moment of warping
t	wall thickness
$T_f$	driving period
I T	transformation matrix
I <sub>SV</sub> T	Same-vename torque
1 <sub>W</sub> 11 V W	displacements of the uncracked beam centroid
$u_{y}, v_{y}, v_{z}$ $u_{y}, u_{y}, u_{z}$	displacements of any point of the beam
U	strain energy of the beam
$U_0$	strain energy of the uncracked element
$U_C$	strain energy due to the presence of a crack
$V_{(i)}$	work of external loads
<b>W</b> <sup>(1)</sup>	vector of nodal displacements of node <i>i</i>
x, y, z x	Carlesian coordinates
χ. Υ, Ζ	coordinates of a point located in the middle line of the cross-section

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