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Research Paper

Arching in geogrid-reinforced pile-supported embankments over silty clay of medium compressibility: Field data and analytical solution





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ABSTRACT

The objective of this study is to improve the understanding of load transfer mechanism of Geogrid-Reinforced Pile-Supported Embankments (GRPS) via a new 3D analytical approach and comprehensive field tests. A full-scale embankment was built over a silty clay of medium compressibility as a part of the Liuzhou-to-Nanning High-speed Railway (LNHR) in China. Six sections of the embankment have been heavily instrumented producing comprehensive data of high quality. Field measurements evidence the existence of soil arching, membrane contribution and ground reaction, phenomena that are all contributing to load transfer mechanism. The new 3D analytical arching model accounts for a triangular arrangement of piles and, unlike existing methods, accounts for all relevant components of load transfer mechanisms. In addition, two key parameters were introduced in the model: an elastoplastic state parameter of soil arching (α) and a coefficient of equivalent uniform stress (β). The former was used to satisfy the load equilibrium in case of partial arching while the latter was adopted to allow possible nonuniform vertical stress acting on the ground surface. The so-called ground reaction method was incorporated in an innovative manner to take into account the reactive support of the subsoil beneath geogrid-reinforced layer when estimating the tension development in the geogrid. Finally, the performance of the proposed model was assessed against several existing models and field measurements. Results showed that the new model presented herein outperforms existing models and satisfactorily predicts both the pile efficiency and tension development within the geogrid.

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1. Introduction

Designing and constructing road and railway embankments over problematic soils, such as soft soils, expansive soils or liquefiable soils, is not trivial because of the excessive total and differential settlements, low bearing capacity, large lateral displacement and slope instability resulting from such soils (e.g. [1-3]). To mitigate these issues, engineers resort to a variety of ground improvement methods such as preloading, stage construction and pile foundations (e.g. [4-6]). Recently, Geogrid-Reinforced and Pile-Supported (GRPS) embankments have gained much popularity due to its inherent short construction time and the efficiency in reducing both vertical and horizontal displacements (see reviews by [7,8]). A GRPS embankment is a complex soil-structure interaction system (see Fig. 1) that efficiently combines vertical piles (floating or end-bearing) with caps and geogrid-reinforced earth platform. GRPS embankments have been adopted as a technical solution for most high-speed railways in China (e.g. [9,10]). Although it is acknowledged that embankment loads are transmitted to the piles by a soil arching effect in the embankment fill and via the geogrid layer [11], the exact process of development of load transfer between embankment mattress and piles is still not well understood. With its comprehensive dataset, this paper will bring new information on this aspect.

When it comes to soil arching, most models were developed for square piles configuration, at the exception of the work by Kempfert et al. [12] who considered a triangular layout for relatively low embankments but effectively produced a 2D design. A large number of models are analytical and based on different simplifying assumptions about load transfer mechanisms (see reviews by [8,11,13–15]). Van Eekelen et al. [16] proposed to organize these

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H L s d_p d_c d_s S_s L_{soil} l_g η r A_s A_p σ_s σ_p P_p T T_f χ and λ α_1, α_2 Re A_e σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r σ_r	embankment height pile length pile cap spacing pile diameter pile cap diameter width of square pile cap pile cap spacing for square arrangement thickness of soil layer elongated lengthof geogridbetween caps normalized net cap spacing radial distance tributary area of soil between piles area of pile cap ground pressure measured on subsoil ground pressure measured on pile caps total force on pile caps uniform tensile force of geogrid tensile force from field test fitting parameters of $k-H/(s - d_c)$ curves angle of load diffusion radius of spatial arching total composite area allocated to each pile an unit area of three adjacent pile caps radial stress	$ar{\sigma}_{sa}$ and $ au_{top}$ and γ K_p α β OCR E_p m ϕ K_g k ε_g δ_{as} δ_s T_a H_f C_c σ_{below} , p l_o	$\bar{\sigma}_{sb}$ equivalent uniform stress acting ongeogrid τ_{bottom} shear stress acting on geogrid unit weight of embankment fill Rankine earth pressure coefficient elastoplastic state parameter coefficient of equivalent uniform soil pressure over-consolidation ratio pile efficiency replacement ratio internal friction angle of fill material tensile stiffness of geogrid ground reaction moduli axial strain of geogrid average pile cap settlement ground settlement measured between two adjacent piles tensile force provided by analytical model final embankment height arching coefficient $\sigma_{_within}, \sigma_{_above}$ radial stress below, within and above spatial arching, respectively characteristic value of live load initial length of geogrid
$\sigma_r \ \sigma_ heta$	radial stress hoop stress		

models in three main categories: (i) limit-state equilibrium models (e.g. [12,16–19]), (ii) frictional models (e.g. [20,21]), and (iii) rigid arch models (e.g. [22]). It has been showed that these approaches give considerably different results in terms of load distribution between piles and surrounding soils [13,23]. In addition, the load transfer mechanisms within GRPS embankment have also been studied via numerical methods, for instance, using finite element method (FEM) (e.g. [24,25]), finite difference method (FDM) (e.g. [3,26]) and discrete element method (DEM) (e.g. [11,27,28]).

Regarding the loading mechanism of geogrids (Fig. 1), several 2D analytical methods have been developed [29–32], and some were extended to three-dimensional cases (e.g. [12,20]). Most of these methods were developed for embankments over soft soils where the upwards ground counter-pressure (illustrated in Fig. 1) on the geogrid is negligible. However, in case of the subsoil of medium compressibility, ignoring this counter-pressure may result in significant overestimation of the required geogrid tensile strength.

Validation of design models requires extensive and good quality data and researchers often employ reduced scale models to validate empirical and numerical approaches [15,20,33–38] while only a limited number of full scale tests have been carried out [9,10,14,39–43]. Full-scale field tests are extremely time-consuming and expensive to conduct and, hence, full-scale data on GRPS are very rare in the literature. Nonetheless, full-scale tests are critical to improve our understanding of GRPS embankments and to validate the existing design methods.

Considering the popularity of equilateral triangular piles configuration in the Chinese high-speed railway network and the extent of the railway network, there is a need for a rigorous 3D triangular design approach coupled to experimental validation in case of high embankments. So far, such model is not available in the literature. In addition, neglecting the soil counter pressure on the geogrid, as it is often done in existing methods, leads to an overdesign of the geogrid, and consequently, an unnecessary over-cost. The present



Fig. 1. Sketch of an embankment reinforced by geogrids and floating piles (a) or end-bearing piles (b).

research will constitute a step towards establishing an innovative and more cost-effective approach. To do so, this paper presents the derivation and validation of a new 3D model for triangular arrangement of piles with a geogrid. The innovation also lies within the fact that, unlike existing methods, the new model accounts for all relevant load transfer mechanisms. These are the progressive development of soil arching, the non-uniform distribution of stress on the ground, a non negligible ground reaction and the progressive development of tension in the geogrid. The penultimate section of the paper will demonstrate how the new model yields much better predictions than existing methods. The validation was made possible by the extensive and high quality data coming from a test embankment of the Liuzhou-to-Nanning High-speed Railway (LNHR, China) where six sections where instrumented [9]. The extensive and rare data presented in this paper will contribute to a better understanding of the load transfer mechanisms in GRPS.

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