







A perturbation method for nonlinear vibrations of imperfect structures: Application to cylindrical shell vibrations

E.L. Jansen *

Faculty of Aerospace Engineering, Delft University of Technology, Kluyverweg 1, 2629 HS Delft, The Netherlands

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Abstract

A perturbation method is used to analyse the nonlinear vibration behaviour of imperfect general structures under static preloading. The method is based on a perturbation expansion for both the frequency parameter and the dependent variables. The effects on the linearized and nonlinear vibrations caused by geometric imperfections, a static fundamental state, and a nontrivial static state are included in the perturbation procedure.

The theory is applied in the nonlinear vibration analysis of anisotropic cylindrical shells. In the analysis the specified boundary conditions at the shell edges can be satisfied accurately. The characteristics of the analysis capability are shown through examples of the vibration behaviour of specific shells. Results for single mode and coupled mode nonlinear vibrations of shells are presented. Parametric studies have been performed for a composite shell. © 2007 Elsevier Ltd. All rights reserved.

Keywords: Perturbation method; Nonlinear vibration; Shell structures

1. Introduction

Investigations in the field of the nonlinear vibration behaviour of structures used to rely on analytical techniques, before the advent of powerful numerical approaches such as the commonly used Finite Element Method (Evensen, 1974). Although the required computational resources are currently available to use the Finite Element Method in combination with numerical time integration, analytical or semi-analytical (i.e. analytical–numerical) approaches remain indispensable to obtain insight in the behaviour of the structure. The starting point for an analytical approach is generally the set of governing differential equations of the specific structure under consideration. Often the spatial dependence of the solution is taken care of by means of a Galerkin-type discretization, while a perturbation technique is used to describe the temporal behaviour (Nayfeh and Mook, 1979).

As a generalisation of such ad hoc analytical approaches for specific structures, Rehfield (1973) introduced a perturbation method analogous to Koiter's initial postbuckling theory (Koiter, 1945) to investigate the

^{*} Tel.: +31 15 27 82592; fax: +31 15 27 85337. *E-mail address*: e.l.jansen@tudelft.nl

Nomenclature

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a_s, b_s, b_{ii} nonlinearity coefficients static state
a_d, b_d, b_{iik} nonlinearity coefficients dynamic state
a_{ijk}, b_{ijkl} nonlinearity coefficients dynamic state, multi-mode analysis
Н
          homogeneous linear functional
L_1
          homogeneous linear functional
L_2, L_{11} homogeneous quadratic functional
          generalized mass operator
M
          generalized applied load
q
          time
t
u
          generalized displacement
ū
          generalized geometric imperfection
u*
          geometric imperfection, nontrivial mode
          generalized strain
\epsilon
          normalized static load parameter
Λ
          linear bifurcation buckling load
\Lambda_c
          generalized stress
          perturbation parameter
\xi_s, \, \xi_d, \, \xi_t perturbation parameter; static, dynamic, imperfect dynamic state
          radial frequency
ω
          linear natural frequency
\omega_c
          linear natural frequency, multi-mode analysis
\omega_{c_I}
Shell analysis (Section 6)
A_{ij}, B_{ij}, D_{ij} stiffness matrices
A_{ij}^*, B_{ij}^*, D_{ij}^* semi-inverted stiffness matrices
\bar{A}_{ij}^*, \bar{B}_{ij}^*, \bar{D}_{ij}^* nondimensional semi-inverted stiffness matrices
          =\sqrt{3(1-v^2)}
          reference Young's modulus
E_{11}, E_{22} Young's moduli orthotropic layer
          fundamental state stress function component
f_0
          first-order stress function components
f_1, f_2
f_{\alpha}, f_{\beta}, f_{\gamma} second-order stress function components
          Airy stress function
F^{(0)}, F^{(1)}, F^{(2)} stress functions for 0th-, 1st-, and 2nd-order state
          shear modulus orthotropic layer
G_{12}
h
          reference shell wall thickness
h_k
          thickness of k-th layer
          shell length
L_{A^*}, L_{B^*}, L_{D^*} linear operators
          nonlinear operator
L_{NL}
M_x, M_y, M_{xy}, M_{yx} moment resultants
          number of full waves in circumferential direction
N_x, N_y, N_{xy} stress resultants
          applied axial load (N_0 = -N_x(x = L))
N_0
          classical buckling load (N_{cl} = (Eh^2)/(cR))
N_{cl}
          applied external pressure
p
          normalized external pressure (\bar{p} = (cR^2)/(Eh^2)p)
\bar{p}
R
          radius of shell
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