

## Effects of wetting and ageing on 1D elasto-viscoplastic behaviour of cement-mixed clay and model simulation

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## Abstract

The combined effects of wetting, the viscous property and ageing on the one-dimensional (1D) compression characteristics of kaolin were evaluated. Two series of special constant-rate-of-strain (CRS) 1D compression tests, with sustained loading at intermediate stages, were performed on kaolin with and without cement-mixing. Most of the air-dried specimens were made partially or fully saturated at intermediate stress levels. The effects of the degree of saturation, the wetting process period, the stress level at which wetting was made, the strain rate during loading and the over-consolidation history applied by compaction during the preparation of the specimens were evaluated. The effects of wetting, viscous property and ageing, observed in the experiments, were significant and complicated. A non-linear three-component elasto-viscoplastic model, that had been modified to take into account the effects of wetting, was further modified to account for the ageing effects. The effects of wetting and ageing were described by a decrease and an increase in the inviscid yield stress at a fixed irreversible strain in the plastic component. The viscous property changed from the basic type, Isotach to TESRA as the ageing effects due to cement-hydration developed. The stress–strain relations and the time histories of stress and strain, during complicated loading and wetting histories applied in the experiments, were successfully simulated by the modified model. © 2012. The Japanese Geotechnical Society. Production and hosting by Elsevier B.V. All rights reserved.

Keywords: Clay; Model simulation; Oedometer tests; Three-component model; Elasto-viscoplastic property; Wetting; Ageing (IGC: D5/E13)

## 1. Introduction

Wetting may induce the significant compression of unsaturated loose soil (e.g., Jennings and Burland, 1962;

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Alonso et al., 1990; Jotisankasa et al., 2007). Since Jennings and Knight (1957) proposed an experimental method to evaluate the collapse deformation during the wetting of soil, a number of models have been proposed to describe the collapse deformation of geomaterials under various loading conditions (e.g., Wheeler et al., 2003; Gallipoli et al., 2003). Although the viscous property is not taken into account in most of these models, its significant effects have been observed not only with saturated clay (e.g., Kawabe et al., 2009), but also with compacted air-dried or unsaturated clay powder (e.g., Li et al., 2004; Deng and Tatsuoka, 2004, 2007). Therefore, it is necessary to properly evaluate the effects of the viscous property on deformation during and after wetting (e.g., De Gennaro et al., 2009). Moreover, the ageing effects on the stress-strain behaviour of natural clays and young cementmixed soil have been studied (e.g., Burland, 1990; Leroueil

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Nomenclature		$\sigma_{v-n}^v$	viscous component of net vertical stress
		$(\sigma_{v-n}^J)_y$	inviscid yield stress in terms of net
α	parameter in the viscosity function		vertical stress
$\alpha_{T2-A}$	angle showing ageing effect	$A_i$	parameters showing ageing effect on $C_c$
$\alpha_{T2-w}$	angle showing wetting effect	$C_{c}$	compression index
β	rate-sensitivity coefficient	$C_{e}^{e}$	compression index of elastic component
$\Delta t_{wet}$	duration of wetting process	$C_{c}^{ur}$	compression index of irreversible component
$\Delta e_m$	change in measured void ratio change at	$C_{c0}^{ur}$ and	d $C_{c1}^{\prime\prime}$ values of $C_c^{\prime\prime}$ when $S_r=0.0$ and 1.0
	wetting stage	$C_r$	recompression index
$\Delta e_{m,f}$	value of $\Delta e_m$ for fully saturated specimen at	$C_{r-A}^{\prime r}$	recompression index during ageing process
	wetting stage	$e_0$	initial void ratio at the start of test
$\Delta e_w$	total void ratio change at wetting stage	$f_{sr}$	function showing the effect of $S_r$ on ageing
$\Delta e_{w,f}$	value of $\Delta e_w$ for fully saturated specimen at	$g_{decay}$	decay function
	wetting stage	$g_v$	viscosity function
έ <sup>e</sup>	elastic component	$h_s$	loading history parameter
$\dot{\varepsilon}^{ir}$	irreversible component	т	parameter in the viscosity function
$\varepsilon_1^{ir}$	initial irreversible strain	$R_{\Delta em}$	ratio of $\Delta e_m$ to $\Delta e_{m,f}$
$\dot{\varepsilon}_{ m r}^{ir}$	parameter in the viscosity function	$S_{rA}$	minimum value of $S_r$ for a full hydration
$\dot{arepsilon}_v$	vertical strain rate		of cement
$\theta$	viscous property type parameter	$t_1$	initial time
$\theta_{end}$	ultimate value of $\theta$	$t_{A,i}$	constant parameters showing ageing effect on
$ ho_c$	density of kaolin particles		$C_c$
$\sigma_{v-n}$	net stress	$t_c$	curing time
$\sigma_{v0}$	initial value of $\sigma_{v-n}$	$t_{CrA-1}$	and $t_{CrA-2}$ constant parameters showing ageing
$\sigma^{f}$	inviscid component		effect on $C_r^{ir}$
$\sigma^v$	viscous component	t <sub>decay</sub>	parameter in function $\theta$
$\sigma_{iso}^{v}$	Isotach type viscous component of stress	$t_p$	time at the start of water percolation
$\sigma_{TESRA}^{v}$	TESRA type viscous component of stress	$\dot{w}_0$	initial water content (in decimal)
$\sigma_{v-n}^{\hat{f}}$	inviscid component of net vertical stress	W <sub>peak</sub>	maximum water content (in decimal)

and Marques, 1996; Tatsuoka et al., 1999, 2008b; Kongsukprasert et al., 2007). The importance of a correct understanding and an accurate evaluation of ageing effects and ageing process in geotechnical engineering practices has been demonstrated by a number of full-scale case histories and related laboratory stress-strain tests on geomaterials.

Di Benedetto et al. (2002, 2005), Tatsuoka et al. (2002, 2008a) and Tatsuoka (2007) showed that a non-linear threecomponent elasto-viscoplastic model (Fig. 1) can properly simulate the viscous effects on the stress–strain behaviour, including creep deformation, and the strain rate effects on the stress–strain relation of a wide variety of geomaterials (i.e., soft clay, sand, gravel, sedimentary soft rock and cementmixed soil) in triaxial and plane strain compression, torsional



Fig. 1. Non-linear three-component model.

and direct shear and one-dimensional compression. Deng et al. (2011) studied the effects of wetting on the stress-strain behaviour of clay powder in 1D compression and modified the model (Fig. 1) to take into account the wetting effects. Extending the above, the present study was performed to evaluate the ageing effects, as well as the wetting effects, on the elasto-viscoplastic behaviour of cement-mixed kaolin powder in 1D compression and to modify the model accordingly. Firstly, the effects of ageing on the 1D compression behaviour were evaluated by wetting compacted and uncompacted cement-mixed kaolin powder from air-dried conditions toward different degrees of saturation at different vertical stress levels, and subsequently, performing monotonic loading (ML) at a constant strain rate. Secondly, the model was modified to describe not only the eventual effects of wetting and ageing on the stress-strain behaviour, but also the process of their development. In so doing, the effects of wetting and ageing were treated as negative and positive effects, respectively, on the inviscid yield stress at a fixed irreversible strain during primary loading. The ageing effects on the viscous property were also considered. The most important contribution of the present study is that, for the first time, the effects of both wetting and ageing on the elastoviscoplastic stress-strain behaviour of soil have been experimentally evaluated and the framework for taking these

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