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Panel reliability assessment for FPSOs

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ABSTRACT

An assessment method is developed for the panel reliability of ship-shaped Floating, Production, Storage and Offloading units (FPSO). Not only axial compressive loads but also internal and external lateral pressures are taken into account in the reliability assessment. Beam-column buckling and flexural-torsional buckling are regarded as two primary failure modes of stiffened panels. Variability of corrosion wastage and material properties are accounted for in modelling the panel's time-dependent ultimate strength. Uncertainty of axial compressive loads induced by hull girder bending is evaluated based on probabilistic characteristics of still-water bending moment (SWBM) and vertical wave-induced bending moment (VWBM). A case study is performed to demonstrate this method and the effects of the lateral pressure, the return period of the extreme value of VWBM, the environmental severity factor, and the corrosion wastage on the panel reliability are investigated. Sensitivity measures for random variables are also carried out.

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1. Introduction

Floating, production, storage and offloading units (FPSOs) are widely utilized in offshore oil and gas fields. A FPSO is operated at a specific location and is usually designed to not sail away even during adverse weather conditions. The structural strength assessment, including the reliability-based strength assessment for FPSOs under severe sea conditions during their service life, is of vital importance.

In the past four decades, considerable work has been conducted on hull girder reliability assessment [1,28,29,33,31,42,11,6]. For panel reliability assessments are much less reported and most of previous work concerns only the failure of deck panels, and thus the applied loads are uniaxial compressive stresses induced predominately by hull girder bending [29,4,12,5]. More information on the state of the art of panel reliability assessment can be found in ISSC reports [3,35].

As an extension of the work of Chen et al. [8] and Chen [6], this paper aims at developing a methodology for panel reliability assessment of FPSOs considering both axial compressive loads and lateral pressures. Beam-column buckling and flexuraltorsional buckling are regarded as two primary failure modes of stiffened panels. Variability of corrosion wastage and material properties are accounted for in modelling panel time-dependent ultimate strength. Uncertainty of axial compressive loads induced by hull girder bending is evaluated based on probabilistic characteristics of still-water bending moment (SWBM) and vertical wave-induced bending moment (VWBM). Lateral pressures imposed on the stiffened panel is calculated based on the external and internal pressures due to sea water, cargo, ballast water, etc. Then, stiffened panels from deck and bottom structures are utilized for a case study to demonstrate the capability of the method developed. The effects of the lateral pressure, the return period of the extreme value of VWBM, the environmental severity factor, and the corrosion effects on the panel reliability are investigated. Sensitivity measures for random variables are also conducted.

2. Ultimate strength of stiffened panel

Ultimate strength of a stiffened panel is evaluated by two primary failure modes, namely, beam-column buckling and flexuraltorsional buckling. The critical buckling stresses for the two failure modes are predicted based on the formulae given in FPI [36].

2.1. Beam-column buckling

The critical buckling stress of a stiffened panel corresponding to the failure mode of beam-column buckling is given by:

$$\sigma_{ca} = \begin{cases} \sigma_E & \sigma_E \leqslant P_r \sigma_y \\ \sigma_y [1 - P_r (1 - P_r) \sigma_y / \sigma_E] & \text{otherwise} \end{cases}$$
(1)







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Nomenclature

σ_a^{exe}	extreme value of the axial compressive stress on a stiff-	d_m	long-term corrosion wastage
		u _w F	Voung's modulus of a material
γ	$=\frac{s\sqrt{\sigma_y/E}}{t_p}$	$F[\mathbf{X}]$	mean matrix of the vector of random variables X
χ	aspect ratio, <i>l/s</i>	f	limit state function
ho	density of sea water	g	gravitational acceleration
σ	scale parameter of Gumbel distribution	h _a	wave-induced internal pressure head, including inertial
λ	shape parameter of Weibull distribution	u	force and added pressure head
α	unit vector of directional cosines	h _{de}	hydrodynamic pressure head induced by waves
I^{\prime}	Warping constant $\simeq m l_{yf} d_w^2 + d_w^3 t_w^3/36$	hs	hydrostatic pressure head in still water
η_a	amplification factor accounting for the secondary stres-	h _t	depth of a tank
_	ses, σ_{L2}	Ie	moment of inertia of longitudinal or stiffener account-
σ_a	axial compressive stress on a sumened panel		ing for the effective width of the plating attached
0 _{ca}	pel	Io	polar moment of inertia of the stiffened panel, excluding
	lici		the associated plating, about the stiffener toe
σ_{cL}	critical buckling stress for the associated plating, corre-	I_x	moment of inertia of the stiffened panel about the <i>x</i> -
	sponding to <i>n</i> -half waves = $\frac{\pi^2 E(n/\alpha + \alpha/n)^2 (t_p/s)^2}{(1-\alpha)^2 (t_p/s)^2}$		axis, through the centroid of the stiffened panel, exclud-
~	torsional flowural critical buckling stress of a stiffened		ing the plating
0 _{ct}		I_y	moment of inertia of the stiffened panel about the y-
	paner		axis, through the centroid of the stiffened panel, exclud-
σ_E	Euler column buckling stress		ing the plating
6	$E[K/2.6+(n\pi/l)^{2}\Gamma+C_{o}(l/n\pi)^{2}/E]$	I_{vf}	$= t_f b_f^3 \frac{1+3(1-2b_1/b_f)^2 d_w t_w/A_s}{12}$
OET	$= \frac{1}{I_o + C_o (l/n\pi)^2 / \sigma_{cL}}$	35	
∇g	vector of partial derivatives of the limit state function g	ĸ	scale parameter of Weibull distribution
	(X) with respect to the design point $\mathbf{x}^{(k)}$	K	St. Venant torsion constant for the stiffened panel's
ε_i	distribution factor around the girth of the installation at	l,	cross section, excluding the associated plating
	location <i>i</i>	к _с	a correlation factor for a specific combined load case
α_i	sensitivity factor of random variable i	k _c	environmental severity factor (FSF) for external pres-
σ_{L1}	primary stresses resulting from hull girder bending	►EPS/EPP	sure starboard/port
σ_{L2}	secondary stresses resulting from bending of large stiff-	k.	distribution factor along the length of the installation
	ened panels between transverse bulkheads	k _l	pressure distribution function
η_{sw}	model uncertainty factor of SWBM	k	load factor
η_w	minimum specified yield stress of a stiffened papel	k,	load factor
Δ_y	total sectional area of a stiffened namel	k _{VBM}	environmental severity factor
A	area of a stiffened namel accounting for the effective	L	length of a FPSO
n _e	width of the plating attached	1	unsupported span of a stiffened panel
a:	effective resultant acceleration at the point considered	L_n	most probable extreme value based on unrestricted
	longitudinal accelerations of tank contents (cargo or		North Atlantic environment with design return period
	ballast)		for the dynamic load parameters
As	area of the stiffener	Ls	most probable extreme value based on site-specific
a_t	transverse accelerations of tank contents (cargo or bal-		environment with design return period for the dynamic
	last)		load parameters
a_v	vertical accelerations of tank contents (cargo or ballast)	l_t	length of a tank
В	breath of a FPSO	m_b	amplification factor
b_1	smaller outstanding dimension of flange with respect to	IVI _{SW}	still-water bending moment (SWBM)
	centerline of web	IVI _W	Vertical wave-induced behaving moment (VWBM) $M_{\rm corresponding to the exceeding probability of 1/N_{\rm correspondence}$
b _e	effective breadth of attached plating in bending for yield	M	M_W corresponding to the exceeding probability of $1/N$
b _f	width of the flange/face plate	n n	number of half-waves which yield the smallest σ_{rr}
b_t	breadth of a tank	N	number of wave cycles
D _{wL}	effective width of the plating	n	lateral pressure imposed on a stiffened panel
C_1	$= 10.75 - \left(\frac{300-L}{100}\right)^{1.5}$ for 90 m $\leq L \leq 300$ m	p _e	external pressures imposed on a stiffened panel
	$= 10.75 \text{ for } 300 \text{ m} \le L \le 350 \text{ m}$	P_f	probability of failure
	$= 10.75 - \left(\frac{L-350}{100}\right)^{1.5}$ for 350 m $\leq L \leq 500$ m	p_i	internal pressure
C_b	block coefficient of a FPSO	P_r	proportional linear elastic limit of a material
C_{dp}	parameter of tank shape	p_t	total nominal pressure imposed on a stiffened panel
<i>C</i> _m	moment adjustment coefficient	$p_{\nu p}$	pressure setting of the pressure/vacuum relief valve
Co	$=Et_p^3/3s$	r _e	radius of gyration of area A_e
C_{ru}	parameter of tank shape	S	longitudinal spacing
C _X	covariance matrix of the vector of random variables X	SM_e	effective sectional modulus of the longitudinal to flange,
C_{θ}	weight coefficient for θ	т	accounting for the effective breadth b_e
C_{ϕ}	weight coefficient for ϕ	I _C	thickness of the flange/face plate
$a(\iota)$	corrosion wastage at time t	ι_f	mickness of the hange/lace plate

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