



Implementation of the Barcelona Basic Model into TOUGH–FLAC for simulations of the geomechanical behavior of unsaturated soils

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ABSTRACT

This paper presents the implementation of the Barcelona Basic Model (BBM) into the TOUGH–FLAC simulator analyzing the geomechanical behavior of unsaturated soils. We implemented the BBM into TOUGH–FLAC by (1) extending an existing FLAC^{3D} module for the Modified Cam–Clay (MCC) model in FLAC^{3D} and (2) adding computational routines for suction-dependent strain and net stress (i.e., total stress minus gas pressure) for unsaturated soils. We implemented a thermo-elasto-plastic version of the BBM, wherein the soil strength depends on both suction and temperature. The implementation of the BBM into TOUGH–FLAC was verified and tested against several published numerical model simulations and laboratory experiments involving the coupled thermal–hydrological–mechanical (THM) behavior of unsaturated soils. The simulation tests included modeling the mechanical behavior of bentonite-sand mixtures, which are being considered as back-fill and buffer materials for geological disposal of spent nuclear fuel. We also tested and demonstrated the use of the BBM and TOUGH–FLAC for a problem involving the coupled THM processes within a bentonite-backfilled nuclear waste emplacement tunnel. The simulation results indicated complex geomechanical behavior of the bentonite backfill, including a nonuniform distribution of buffer porosity and density that could not be captured in an alternative, simplified, linear-elastic swelling model. As a result of the work presented in this paper, TOUGH–FLAC with BBM is now fully operational and ready to be applied to problems associated with nuclear waste disposal in bentonite-backfilled tunnels, as well as other scientific and engineering problems related to the mechanical behavior of unsaturated soils.

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1. Introduction

The Barcelona Basic Model (BBM) is a geomechanical constitutive model for capturing the elasto-plastic behavior of unsaturated soils. The model was first developed and presented in the early 1990s as an extension of the Modified Cam–Clay (MCC) model to unsaturated soil conditions (Alonso et al., 1990). The model can describe many typical features of unsaturated-soil mechanical behavior, including wetting-induced swelling or collapse strains, depending on the magnitude of applied stress, as well as the increase in shear strength and apparent preconsolidation stress with suction (Gens et al., 2006).

In this paper, we present the implementation of the BBM into a coupled multiphase fluid flow and geomechanical simulator called TOUGH–FLAC (Rutqvist et al., 2002; Rutqvist, 2010). The TOUGH–FLAC simulator is based on the sequential coupling of a finite-difference geomechanical code, FLAC^{3D} (Itasca, 2009) and a finite-volume, multiphase fluid flow code, TOUGH2 (Pruess et al., 1999).

One great advantage of this approach to coupled-processes modeling is that both TOUGH2 and FLAC^{3D} are being continuously developed and widely used, and therefore contain many constitutive and equation-of-state modules that can be readily applied to a wide range of scientific and engineering problems. In this case, we start with the existing MCC module in FLAC^{3D}, which we then extend and modify to model the geomechanical behavior of unsaturated soil conditions within the framework of the BBM.

We implemented a thermo-elasto-plastic version of the BBM, in which the soil strength depends on both suction and temperature, and includes features for expansive (swelling) clay (Gens, 1995). Fig. 1 presents the three-dimensional yield surface in $p' - q - s$ space and $p' - q - T$ space, where p' is the net mean stress (i.e., total stress minus gas-phase pressure), q is the deviatoric stress (or shear stress), s is suction and T is temperature (Gens, 1995). Under water-saturated conditions ($s=0$), the yield surface corresponds to the MCC ellipse (Roscoe and Burland, 1968) and the size of the elastic domain increases as suction increases. The rate of increase, represented by the loading-collapse (LC) curve, is one of the fundamental characteristics of the BBM (Gens et al., 2006). Moreover in the thermo-elasto-plastic version of the BBM, the size of the yield surface decreases with temperature (Fig. 1). We implemented

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Nomenclature			
c_a, c_b	constants used in the calculation of the plastic multiplier [Pa]	J_2	second invariant of the effective deviatoric-stress tensor [Pa]
e	void ratio [–]	J'_2	second invariant of the deviatoric-strain tensor [–]
\mathbf{e}	deviatoric-strain tensor [–]	v	specific volume [–]
D_v	effective molecular diffusion coefficient of vapor [m ² /s]	v_i, v_{ref}	specific volume at initial and reference stress states [–]
f_{LC}	yield surface for loading–collapse in the BBM [Pa]	V	volume [m ³]
g_{LC}	plastic potential for loading–collapse in the BBM [Pa]	V^s	volume of solid phase [m ³]
g_y	parameter describing the shape of the yield surface in the BBM [–]	V^p	volume of pores [m ³]
G	shear modulus [Pa]	α_a	nonassociativity parameter in flow rule in the BBM [–]
\mathbf{I}	identity tensor (all components 0, except diagonals which are 1) [–]	α_B	Biot's effective stress coefficient [–]
k	permeability [m ²]	α_{ps}	parameter relating elastic compressibility to suction in the BBM [–]
k_r	relative permeability [–]	α_{sp}	parameter relating κ_{sp} to net mean stress in the BBM [–]
k_s	parameter describing the increase of cohesion with suction in the BBM [–]	α_{ss}	parameter relating κ_{sp} to suction in the BBM [–]
K	bulk modulus [Pa]	α_0, α_2	parameters that relate elastic volumetric strain and temperature changes in the BBM [°C ⁻¹]
K^s	bulk modulus for suction-induced volumetric strain in the BBM [Pa]	α_T	linear thermal expansion coefficient (equivalent to α_0) [°C ⁻¹]
LC	loading–collapse yield surface in the BBM [–]	β_λ	parameter for the increase of soil stiffness with suction in the BBM [Pa ⁻¹]
LC_i	loading–collapse yield surface at initial conditions in the BBM [–]	β_{sw}	moisture swelling coefficient in an LE swelling model [–]
M	slope of the critical state line in the BBM [–]	ϵ_v	volumetric strain (= $\epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$) positive for contraction [–]
p	total mean stress (Eq. (2)), compression positive [Pa]	$\boldsymbol{\epsilon}^s$	suction strain tensor [–]
p'	net mean stress (Eq. (3)), compression positive [Pa]	$\boldsymbol{\epsilon}$	total strain tensor [–]
p^c	a reference stress state for $v - p'$ relation in virgin states in the BBM [Pa]	$\boldsymbol{\epsilon}^e$	elastic strain tensor [–]
p_{est}'	estimated mean stress in FLAC ^{3D} elastic predictor-plastic corrector algorithm [Pa]	$\boldsymbol{\epsilon}^p$	plastic strain tensor [–]
p^g	gas phase, g , pressure [Pa]	$\boldsymbol{\epsilon}^T$	thermal strain tensor [–]
p^l	liquid phase, l , pressure [Pa]	$\epsilon_v^e, \epsilon_q^e$	elastic volumetric and deviatoric strains [–]
p^p	pore pressure [Pa]	$\epsilon_v^p, \epsilon_q^p$	plastic volumetric and deviatoric strains [–]
p_s	tensile strength in the BBM [Pa]	$\epsilon_v^T, \epsilon_s^T$	thermal and suction-induced volumetric strains [–]
p_{s0}	tensile strength at saturated conditions in the BBM [Pa]	ϕ	porosity [–]
p_0	net mean yield stress at current suction and temperature in the BBM [Pa]	κ_{ps}	compressibility parameter for elastic $v - p'$ in the BBM [–]
p_{ref}	reference stress state for relating elastic compressibility to suction in the BBM [Pa]	κ_{ps0}	initial (zero suction) slope for elastic $v - p'$ in the BBM [–]
p_{0T}^*	net mean yield stress for saturated conditions at reference temperature in the BBM [Pa]	κ_{sp}	compressibility parameter for suction-induced elastic strain in the BBM [–]
r_λ	parameter defining the maximum soil stiffness associated with an LC yield in the BBM [–]	κ_{sp0}	κ_{sp} at reference stress P_{ref} and zero suction [–]
s	suction [Pa]	λ_{ps}	compressibility parameter in virgin soil states at suction s in the BBM [Pa]
\mathbf{s}	deviatoric-stress tensor [Pa]	λ_{ps0}	Slope of $v - p'$ relation in virgin soil states at zero suction in the BBM [Pa]
S_l	liquid phase saturation [–]	\mathcal{A}	plastic multiplier [–]
T_0	reference temperature for temperature-dependent cohesion in the BBM [°C]	θ	Lode's angle [°]
T	temperature [°C]	ρ_d	dry-density, ρ_d [kg/m ³]
q	deviatoric (von Mises) stress [Pa]	ρ_s	parameter that relates cohesion to temperature in the BBM [–]
q_{est}	estimated deviatoric stress in FLAC ^{3D} elastic predictor-plastic corrector algorithm [Pa]	$\sigma_1, \sigma_2, \sigma_3$	principal compressive stress components [Pa]
		$\boldsymbol{\sigma}$	total stress tensor [Pa]
		$\boldsymbol{\sigma}'$	effective stress tensor [Pa]
		ν	Poisson's ratio [–]

the BBM into TOUGH–FLAC by (1) extending an existing MCC module within the framework of the FLAC^{3D} User Defined Model (UDM) capability and (2) adding computational routines for suction-dependent strains and net stress in unsaturated soils.

The thermo-elasto-plastic version of the BBM is also part of the CODE_BRIGHT finite element code at the University of Cataluña, Barcelona (CIMNE, 2002; Olivella et al., 1996). It was recently

successfully applied to model the coupled thermal–hydrological–mechanical (THM) behavior of an unsaturated bentonite clay associated with the FEBEX in situ heater test at the Grimsel Test Site, Switzerland (Gens et al., 2009). The BBM has also been applied to other types of bentonite-sand mixtures such as MX-80, considered as an option for an isolating buffer in the Swedish KBS-3 repository concept (Kristensson and Åkesson, 2008a, b).

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