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Hydrodynamic characteristics and geometric properties of plunging and spilling breakers over impermeable slopes

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ABSTRACT

The two-phase flow CFD model REEF3D has been used for modeling waves breaking over a sloping seabed for a spilling and a plunging breaker. This model is based on Reynolds-averaged Navier–Stokes (RANS) equations with the level set method (LSM) for the free surface and *k*–ω model for turbulence. First, the characteristics and geometric properties of plunging breaking waves with different offshore wave steepnesses over slopes are examined and discussed. The study further explores the hydrodynamic characteristics of spilling and plunging breakers in terms of the wave height evolution and attenuation, horizontal and vertical velocity, free surface profile evolution, and the geometric properties during the development of the breaking process. The numerical results show a good agreement with experimental data in terms of free surface elevation and horizontal and vertical velocity for the spilling and plunging breakers. Results of numerical simulations describing the physical flow characteristics such as the formation of the forward overturning water jet, air pocket, splash-up, and the secondary wave during the breaking process are presented for both cases. For both cases, the physical flow process is found to have similar flow features, but the breaking process occurs at significantly different scales.

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1 **1. Introduction**

 Wave breaking is a two-phase flow process composed of air and water, which transforms the large scale deterministic irrotational flow into rotational flow resulting in turbulence and vortices of dif- ferent types and scales. The wave breaking process in shallow wa- ters naturally influences many physical processes such as wave en- ergy dissipation, air–sea interaction, wave–structure interaction, rip current, cross-shore and along-shore currents, sediment transport, shoreline evaluation. Breaking waves are strongly influenced by the local wave parameters and seabed slope, and are described by four [different types: spilling, plunging, collapsing and surging \(Galvin,](#page--1-0) 1968). Breaking waves exert significant hydrodynamic loading on offshore platforms and foundations of offshore wind turbines in intermediate and shallow water. A recent feasibility study on the deployment of offshore wind turbines on Thornton bank outside the Belgian coast shows that hydrodynamic forces from plunging breaking waves govern the design criteria of a truss substructure

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[\(Alagan Chella et al., 2012\)](#page--1-0). Understanding the breaking process re- 18 mains a challenge since both measurements and simulations are ex- 19 tremely intricate. 20

[Svendsen et al. \(1978\)](#page--1-0) divided the surf zone from inception to bro- 21 ken waves into three regions: outer, inner and run-up region. In the 22 outer region, waves undergo drastic changes in the shape and the 23 flow features, i.e. the flow pattern changes from irrotational flow to 24 rotational flow. As the wave propagates farther shoreward, the wave 25 motion becomes turbulent with low frequency components leading 26 to the formation of wave rollers in the inner region. The region clos- 27 est to the shore is the run-up region. The two-dimensional effects and 28 longitudinal variations are more pronounced when waves approach 29 the breaking point. On the other hand, the three-dimensional effects 30 become more significant just after breaking and the flow becomes 31 highly turbulent where waves undergo drastic changes in the de-
32 terministic flow characteristics. Therefore, three-dimensional effects 33 and the surface tension effects need to be considered for a better de- 34 scription of air entrainment during the breaking process and the tur-
35 bulent flow characteristics in the surf zone. The present study focuses 36 on the physical process up to the inner breaking region where the 37 three-dimensional effects are minimal, i.e. the large-scale changes in 38

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Fig. 1. (a) Definition sketch of local steepness and asymmetry parameters following [Kjeldsen and Myrhaug \(1978\).](#page--1-0) (b) Schematic of formation of plunger vortex and surface vortex during breaking following [Basco \(1985\).](#page--1-0)

 the incident deterministic flow pattern. The wave breaking process primarily depends on the following parameters: local water depth 41 (*d*), offshore wave steepness (H_0/L_0 , where H_0 and L_0 are wave height and wave length, respectively in deep water) and sea bed slope (*m*). The wave characteristics and the seabed slope are key factors in deter- mining the breaker type [\(Iversen, 1952; Galvin, 1968; Battjes, 1974\)](#page--1-0). [Battjes \(1974\)](#page--1-0) described the breaker types based on the surf similarity 46 parameter ($\xi_0 = \frac{m}{\sqrt{H_0/L_0}}$), which is a function of the wave steepness 47 in deep water (H_0/L_0) and the seabed slope (*m*). For $\xi_0 < 0.5$, spilling breakers develop for waves of high steepness over mild slopes with the formation of white water foam or a small overturning water jet 50 at the wave crest. For $0.5 < \xi_0 < 3.3$ corresponding to waves of low steepness propagating over moderate seabed slopes, waves break as plunging breakers with the larger forward overturning jet at the wave 53 crest. Surging or collapsing breakers occur for $\xi_0 > 3.3$. Many lab- oratory experiments have been performed to obtain more insights into the breaking wave geometric, kinematic, dynamic and turbu- [lent characteristics in deep and shallow waters, such as](#page--1-0) Kjeldsen and Myrhaug (1978), [Adeyemo \(1968\),](#page--1-0) [Stive and Wind \(1982\),](#page--1-0) Miller (1987), [Nadaoka et al. \(1989\),](#page--1-0) [Smith and Kraus \(1990\), and](#page--1-0) Ting and Kirby (1994). [Kjeldsen and Myrhaug \(1978\)](#page--1-0) proposed steepness and asymmetry parameters from zero-downcross analysis to describe the prominent asymmetry features of a wave that approaches breaking: 62 crest front steepness (ε), crest rear steepness (δ), the vertical asym-63 metry factor (λ) and the horizontal asymmetry factor (μ) as defined in Fig. 1(a). A detailed experimental study by [Ting and Kirby \(1994,](#page--1-0) [1995,](#page--1-0) [1996\)](#page--1-0) investigated the dynamics of surf zone turbulence under waves breaking over a sloping seabed using fibre-optic laser-Doppler anemometer (LDA) technique. They addressed the basic differences in the turbulent characteristics and turbulence production mecha- nisms between spilling and plunging breakers. A number of stud- ies have reviewed the wave evolution, flow properties and physical [characteristics of waves breaking in deep and shallow water](#page--1-0) Cokelet (1977), [Peregrine \(1983\),](#page--1-0) [Basco \(1985\),](#page--1-0) [Banner and Peregrine \(1993\),](#page--1-0) and [Perlin et al. \(2013\).](#page--1-0) Importantly, most numerical studies have addressed the deformation of solitary waves during the breaking 74 process in shallow waters including [Emarat et al. \(2012\)](#page--1-0) and Mo et al. 75 [\(2013\). Though a very few numerical studies have investigated the](#page--1-0) 76 periodic waves breaking in shallow waters. 77

The wave transformation process in the surf zone is well repre- 78 sented by the Navier-Stokes equations and a direct solution of these 79 equations is extremely complicated [\(Lemos, 1992\)](#page--1-0). With the advance- 80 ments in the development of computational fluid dynamics (CFD), 81 a numerical model that solves the Navier–Stokes equations coupled 82 with a free surface capturing scheme is capable of solving the com-
83 plex free surface flow problem and details of the fluid flow proper- 84 ties can be obtained. The first numerical investigation of free surface 85 flows by directly solving the Navier–Stokes equations was demon- 86 strated by [Harlow and Welch \(1965\).](#page--1-0) A class of computational meth- 87 ods based on the Reynolds-averaged Navier-Stokes (RANS) equations 88 was first proposed by [Lemos \(1992\)](#page--1-0) together with the volume-of- 89 fluid method (VOF) and the $k - \epsilon$ turbulence model to simulate break- 90 ing waves in shallow water. Several studies attempted to model the 91 breaking process using a single-phase flow model (Lin and Liu, 1998; 92 [Bradford, 2000; Zhao et al., 2004; Christensen and Deigaard, 2001\).](#page--1-0) 93 The major inadequacies of the single-phase flow models to represent 94 the complete wave breaking process are that they do not account 95 for the air phase, the constant pressure assumption in air and the 96 associated boundary conditions at the free surface. Therefore, this 97 model cannot represent the complex air–water interaction, which 98 has a prominent role in the process. Hence, two-phase flow mod- 99 [els are crucial to model the wave breaking process, such as](#page--1-0) Hieu 100 et al. (2004), [Christensen \(2006\),](#page--1-0) [Lubin et al. \(2006\),](#page--1-0) Moraga et al. 101 (2008), [Wang et al. \(2009\),](#page--1-0) [Shi et al. \(2010\),](#page--1-0) [Ma et al. \(2011\),](#page--1-0) Jacobsen 102 et al. (2012), [Xie \(2013\),](#page--1-0) [Alagan Chella et al. \(2015a\) and](#page--1-0) Alagan Chella 103 et al. (2015b). [Alagan Chella et al. \(2015a\) used the present numeri-](#page--1-0) 104 cal model to simulate spilling breakers over slopes. The authors com- 105 pared the numerical results to the experimental data for the spilling 106 breaker case in order to validate the numerical model. Moreover, the 107 main aim of the study was to investigate the effects of water depth, 108 offshore wave steepness, and beach slope on the characteristics and 109 geometric properties of spilling breakers over slopes. 110

The present numerical study uses the incompressible Reynolds- 111 averaged Navier–Stokes equations based numerical wave tank. Un- 112 like most of the previous numerical studies on breaking waves, in the 113 current numerical model, different approaches have been proposed 114 for describing the computational grid architecture and discretization 115 schemes. The employment of the Cartesian grid facilitates to imple- 116 ment higher order spatial and temporal discretization schemes that 117 provide very good numerical accuracy and stability. Particular atten- 118 tion has been given to achieve a more accurate representation of free 119 surface waves in order to avoid the unrealistic damping at the free 120 surface. Several numerical studies that are aimed at modeling the 121 surf zone hydrodynamics have shown quite good results, but far too 122 little attention has been paid to investigate the evolution of the free 123 surface profile and the prominent flow features during the breaking 124 process. In the hydrodynamic load assessment point of view, the evo- 125 lution of free surface profiles, wave height and changes in velocities 126 and geometric properties associated with the initial breaking process 127 are important for the modeling of breaking wave forces. Meanwhile, 128 there have been limited studies on these hydrodynamic characteris- 129 tics relevant to the load assessment parameters in shallow waters. 130

The main purpose of the present paper is to investigate the hydro- 131 dynamic and geometric properties of plunging breakers over slopes 132 with the two-phase flow CFD model REEF3D and compare them with 133 the spilling breakers [\(Alagan Chella et al., 2015a\)](#page--1-0). Comparison with 134 similar results obtained for spilling breakers in Alagan Chella et al. 135 [\(2015a\) are also discussed. First, the study assesses the characteristics](#page--1-0) 136 and geometric properties of plunging breaking waves of different off- 137 shore wave steepnesses over different slopes. This has been accom- 138 plished by examining the breaking characteristics such as breaker 139

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