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#### **Research** Paper

# Comparison of Press-Replace Method and Material Point Method for analysis of jacked piles

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#### 1. introduction

Pile installation using dynamic driving methods is associated with undesirable environmental effects such as noise, vibration and pollution. Therefore, pile jacking (pressing) has become attractive due to the environmental advantages that it has over conventional driving methods [1]. In addition to the environmental advantages, it is possible to estimate the ultimate load capacity of jacked piles during pile installation based on the measured jacking load [2]. Jacked piles, in their initial application, were mainly used to underpin existing foundations to increase their capacity and decrease their settlement [3]. Nowadays, there is an increasing trend in using jacked piles as foundations of new structures, in particular, in urban environment where minimizing the noise and vibration due to construction activities is desirable. Due to the tendency in using jacked piles, many researchers have studied jacked piles using experimental [2,4-10] and computational methods [1,11–13]. Furthermore, the simulation of jacked pile installation is a necessary and beneficial step towards simulating the installation of driven piles.



In this study, installation of jacked piles in sand is simulated using Press-Replace Method (PRM) and Material Point Method (MPM) and the results are compared together. This comparison is important because a realistic and yet efficient simulation of installation of jacked piles is an appealing step towards the design and analysis of this type of displacement piles. It is shown that PRM as a method that is founded on small-strain finite element method can produce pile and soil responses that are in a promising agreement with those of MPM which is a finite-deformation analysis method.

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Realistic simulation of the installation process is a key step in analyzing the behavior of jacked piles. In the past years, number of researchers have focused on simulating the whole installation process using large-deformation numerical analysis methods such as Arbitrary Eulerian–Lagrangian (AEL) method [14,15] and its derivation, namely, Coupled Eulerian–Lagrangian (CEL) method [16,17], adaptive remeshing technique [11], and most recently Material Point Method (MPM) [18,19]. Besides finite-deformation analysis methods, a simpler method entitled Press-Replace Method (PRM) has been successfully used for simulation of jacked pile installation using small-deformation theory [20,21].

MPM has recently gained attentions in simulating largedeformation boundary and initial value problems in geotechnical engineering. Despite its promising performance, MPM is computationally expensive and relatively complicated which decrease its attraction for practice engineers who look for practical and straight-forward methods in a daily engineering practice. PRM, on the other hand, is a simple method that is based on smalldeformation theory, which has been used solely for simulation of penetration problems such as pile jacking and cone penetration. The simplicity of PRM enables an engineer to model the installation process of jacked piles as a staged construction process by any finite element code. The purpose of this study is to compare PRM and MPM for numerical simulation of jacked piles during installation and operation. Such a comparison shows if the PRM can be relied upon for the analysis of jacked piles. It also reveals







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$\alpha$ hypoplasticity parameter that determines the depen- G soil shear modulus	
dency of peak friction angle with respect to the relative $G_i$ interface shear stiffness	
density $\gamma_d$ soil dry unit weight	
$\beta$ hypoplasticity parameter that determines the depen- $h_s$ granular hardness, determines the inclinat	on of the
dency of soil stiffness with respect to the relative void ratio limits in the hypoplastic model	
density $K_0$ coefficient of lateral earth pressure at rest	
<i>B</i> pile diameter <i>L</i> pile length	
$B_R$ intergranular strain parameter of the hypoplastic model $m_R$ intergranular strain parameter of the hypopla	stic model
$C_c$ coefficient of curvature $m_T$ intergranular strain parameter of the hypopla	stic model
<i>C<sub>u</sub></i> coefficient of uniformity <i>n</i> hypoplasticity parameter that determines the	curvature
<i>D</i> <sub>50</sub> mean particle diameter of the void ratio limits	
$D_R$ relative density $Q_{sL}$ limit shaft capacity	
$\delta_c$ critical-state interface friction angle $r$ radial distance from the pile center	
$\delta$ interface friction angle $R_i$ interface reduction factor	
<i>e</i> void ratio $R_{max}$ intergranular strain parameter of the hypopla	stic model
$e_{max}$ maximum void ratio $\sigma_{rr}$ radial stress	
$e_{min}$ minimum void ratio $\sigma_{zz}$ vertical stress	
$e_{d0}$ minimum reference void ratio in the hypoplastic model $\sigma_{rz}$ shear stress	
$e_{c0}$ reference void ratio at the critical state in the hypoplas- $t_s$ thickness of soil slices in the PRM analyses	
tic model $u_r$ radial displacement	
$e_{i0}$ maximum reference void ratio in the hypoplastic model $u_z$ vertical displacement	
$E_p$ Young's modulus of the pile material $v$ soil Poisson's ratio	
$E_{oed,i}$ interface bedometric sumess $v_i$ interface Poisson's fatto	
$\phi$ soli infinition angle w pine head settlement	stic model
$\varphi_c$ childen state include aligner $\chi$ intergradual strain parameter of the hypopla	suc model
$\varphi_p$ peak including angle $\psi$ solid underly angle	
os specific gravity 2 ucptil	

the differences that exist between PRM and a more sophisticated method of pile installation simulation, namely MPM. For simplicity, this paper only focuses on the single-stroke jacking as an initial step in simulating the multi-stroke jacking of piles.

#### 2. Analysis methods

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#### 2.1. Material Point Method

The Material Point Method (MPM) can be viewed as an extension of the Particle-In-Cell method (PIC) and was initially applied to fluid dynamic problem by Harlow [22]. Later on, Brackbill and his co-workers [23] developed the so-called fluid-implicit particle (FLIP) method, that is a PIC formulation, in which the particles carry all physical properties of the continuum. FLIP uses adaptive meshing which is able to model complex geometries and achieves better accuracy than does PIC. In 1994, the FLIP method was extended to adapt into solid mechanics by Sulsky et al. [24]. In the extended method, the weak formulation and the discrete equation are consistent with the finite element method (FEM). Furthermore, the constitutive equation is applied at each single particle, which allows the method to handle the history-dependent material behavior. In



**Fig. 1.** The MPM solution algorithm: (a) initialization step, (b) incremental deformation (Lagrangian step) and  $(c_1)$  resetting the mesh or  $(c_2)$  redefining a new mesh (convective step).

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