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Overall buckling behaviour and design of high-strength steel welded section columns



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ABSTRACT

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Practical use of high-strength (HS) steel in contemporary construction has become one of the most important design solutions. Loading capacities of columns may benefit enormously from the HS steel, whilst their overall buckling behaves differently compared with conventional mild (CM) steel columns due to varying effects of initial imperfections and inelastic properties of the HS steel materials. Despite a number of investigations regarding the HS steel columns being undertaken, there is lack of research focused on variation of HS steel grades and their effects. To deepen understanding of overall buckling behaviour of the HS steel columns, a comprehensive review of an extensive body of column test data available in the literature is carried out in the present paper, based on which a three-dimensional finite element (FE) model developed herein is validated. Parametric analyses are subsequently undertaken with various HS steel grades, welded cross-sectional geometric parameters, slenderness values and initial imperfections being involved. The FE analysis results are also compared with calculation values in accordance with national standards. It has been demonstrated that with an increase of the grade of HS steel, effects of imperfections decrease whilst that of Y/T ratios are rather limited; reduction effects on the overall buckling strength become less severe, and therefore higher column curves available in current national standards may be selected and imperfection factors in the alternative column curve equations proposed herein descend accordingly. In addition, new theoretical column curves based on Perry-Robertson formula are developed by introducing imperfection parameters independent on the steel strength.

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1. Introduction

High-strength (HS) structural steel has attracted increasing attention from both practical engineers [1–3] and academic researchers [4–6], and their usage in contemporary steel construction has become an important design solution. The most essential attribute of such advanced steel in terms of higher yield strength (no less than 460 MPa) compared with conventional mild (CM) steel is particularly beneficial to steel columns subjected to compression. Loading capacities of the columns may be significantly improved, whilst their overall buckling behaviour is quite distinct from CM steel columns. With an increase of the yield strength of HS steel, compressive residual stresses within cross-sections, of which ratio to the yield strength becomes markedly lower [7,8], possess much less severe effects on the overall buckling behaviour; whilst initial bending-induced stresses may also result in reduced effects [9] because of their descending ratios to the yield strength of steel. As a consequence, research and development of robust design methodologies on overall buckling behaviour of HS steel columns are of great significance for providing economical but safe design solutions.

Despite a number of experimental programmes of HS steel columns hitherto having been undertaken and reported in the literature, they focused generally on some individual steel grade without considerations of its variation in the analyses of buckling mechanism as well as in the development of design methods. The very first column tests on HS steel seem to be that by Usami and Fukumoto [10] in 1982, in which five welded box section columns excluding stub ones fabricate from HT80 (690 MPa) HS steel were tested. Rasmussen and Hancock [11] undertook 11 welded box and I-shaped column tests for BISALLOY 80 (690 MPa) HS steel in 1995. After 2010, the authors investigated experimentally and numerically the overall buckling behaviour of four S690 HS steel columns and four S960 ones with end restraint [12], as well as 12 Q460C HS steel welded section columns [13] and six Q960 ones [14]. Similarly, Wang et al. carried out 12 Q460 HS steel welded section column tests as well as corresponding finite element (FE) analyses [15,16], and Zhou et al. [17] designed six Q460 HS steel welded H-shaped section columns with their overall buckling behaviour being investigated experimentally ad numerically. More recently, Li et al. [18–19] tested 12 Q690 HS steel welded box and I-section columns, with FE parametric analyses being conducted. Chung et al. [20] experimentally investigated seven Q690

Table	1
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Detailed information of HS steel column test specimens available in literature.

Specimen	Cross-sectional geometry (mm)	L_0 (mm)	<i>e</i> (mm)	$f_{\rm y}({ m MPa})$	$P_{u \cdot t}$ (kN)	λ_n	φ	$P_{u \cdot FEA}$ (kN)	$\frac{P_{u \cdot \text{FEA}}}{P_{u \cdot t}}$
0.05.00[40]	P420.00 0.00	1000.0	1.10	744.0	2112.0	0.054	0.050	01.17.0	1.015
S-35-22 [10]	$B139.00 \times 6.00$	1880.0	1.18	741.0	2112.0	0.654	0.853	2147.3	1.017
S-50-22 [10]	B138.98 × 6.01	2690.0	0.85	741.0	1/98.0	0.967	0.740	1658.1	0.922
K-50-22 [10]	$B139.00 \times 6.00$	2090.0	0.83	741.0	1622.0	0.930	0.745	1506.3	1.022
K-00-22 [10]	B138.98 × 6.01	2720.0	0.00	741.0	1299.0	1.210	0.594	1327.8	1.022
EK-50-22 [10]	B138.98 × 6.01	2090.0	11.32	741.0	1220.0	0.932	0.558	1221.5	1.001
BIISUC [11]	B98.90 × 5.00	1149.0	0.50	705.0	1174.0	0.562	0.911	10704	0.990
BIIDUE [11]	B97.50 × 4.95	1050.0	2.10	705.0	1137.0	0.570	0.904	1076.4	0.947
B1950C [11] B1050E [11]	$B98.22 \times 4.90$	1950.0	0.50	705.0	1078.0	0.960	0.849	902.8	0.893
B3450C [11]	$B_{33,34} \times 4.57$ B100 14 \times 4.97	3451.0	0.40	705.0	920.0 469.0	1.664	0.719	478.6	1 020
B3450F [11]	$B100.14 \times 4.57$ $B99.78 \times 4.94$	3451.0	2 90	705.0	438.0	1.670	0.340	442.2	1.020
11000C [11]	$H_{155,40} \times 14150 \times 7.70 \times 7.70$	1000.0	0.70	660.0	2092.0	0.545	0.952	1965.0	0.030
11000E [11]	$H157.14 \times 141.10 \times 7.67 \times 7.71$	1000.0	1 30	660.0	2192.0	0.550	0.991	1923.1	0.877
11650C [11]	$H156.90 \times 141.50 \times 7.70 \times 7.66$	1649.0	0.40	660.0	1751.0	0.896	0.800	1739 5	0.993
11650E [11]	$H158.42 \times 141.50 \times 7.71 \times 7.75$	1649.0	1.00	660.0	1682.0	0.900	0.762	1683.7	1.001
I2950E [11]	$H157.50 \times 140.30 \times 7.75 \times 7.74$	2950.0	2.00	660.0	745.0	1.627	0.337	795.3	1.067
S1-690-1300 [12]	$I 120.5 \times 100.2 \times 10.0 \times 8.3$	1216.8	4.69	799.0	1857.9	0.477	0.821	2028.0	1.092
S2-960-1300 [12]	$189.5\times79.5\times7.9\times6.1$	1103.7	8.73	962.5	1368.4	0.625	0.831	1268.6	0.927
S3-690-2700 [12]	$I~121.1\times100.0\times10.0\times8.0$	2385.0	8.08	799.0	1656.5	0.927	0.739	1449.7	0.875
S4-960-2700 [12]	I 120.0 \times 99.2 \times 10.1 \times 7.9	2384.9	11.85	996.0	2099.6	1.066	0.756	1487.2	0.708
S5-690-3600 [12]	I 79.8 \times 70.4 \times 6.0 \times 6.1	2345.5	34.73	740.3	306.4	1.356	0.328	283.7	0.926
S6-960-3600 [12]	$I~95.3\times79.6\times7.8\times6.1$	2673.3	15.32	962.5	434.6	1.424	0.260	564.9	1.300
S7-690-3600 [12]	$1~59.7\times49.9\times5.1\times5.2$	2015.1	39.44	783.3	136.7	1.613	0.228	148.6	1.087
S8-960-3600 [12]	$159.9\times59.2\times6.1\times6.2$	2086.8	17.97	1019.8	210.0	1.919	0.202	226.1	1.077
B1-460 [13]	B152.0 × 10.92	1080.2	5.24	531.9	3129.7	0.300	0.955	2944.5	0.941
B2-460 [13]	B141.1 × 14.83	1261.1	6.68	492.3	3642.3	0.374	0.988	3641.9	1.000
B3-460 [13]	B121.5 × 12.67	1549.4	1.44	492.9	2185.6	0.532	0.804	1961.7	0.898
B4-460 [13]	$B102.4 \times 11.04$	1782.4	2.80	531.9	1503.8	0.760	0.701	1546.9	1.029
B5-460 [13]	$B102.2 \times 10.81$	2279.8	11.89	531.9	930.6	0.972	0.443	899.1	0.966
11-400 [13]	$1111.7 \times 132.1 \times 10.96 \times 11.37$	2571.4	3.13	231.9	1205.4	0.909	0.597	1290.8	1.025
H1-400 [15]	$H_{209,4} \times 210.0 \times 14.60 \times 15.02$	1009.5	0.07	492.5	4407.2 2722.0	0.552	0.707	2010.9	1 022
H3_460 [13]	$H141.0 \times 179.7 \times 13.10 \times 12.90$ $H150.2 \times 151.5 \times 11.08 \times 11.35$	1512.1	2.11	492.5 531.0	1008.6	0.439	0.757	107/1	0.022
H4-460 [13]	$H150.2 \times 151.3 \times 11.00 \times 11.05$ $H151.1 \times 151.2 \times 11.02 \times 11.07$	1815.1	3.90	531.9	1842.4	0.077	0.770	1878.0	0.568
H5-460 [13]	$H1112 \times 1319 \times 10.76 \times 11.34$	2026.0	2.28	531.9	1398.8	1 001	0.669	1230.8	0.880
H6-460 [13]	$H149.4 \times 150.3 \times 11.02 \times 11.09$	1315.2	3.06	531.9	2437.8	0.584	0.956	2122.9	0.871
B1-960 [14] ^a	B142.6 × 13.99	1878.6	25.88	973.2	3779.5	0.775	0.540	3711.2	0.982
B2-960 [14]	B141.6 × 13.94	2879.8	3.13	973.2	4063.9	1.196	0.587	3704.1	0.911
B3-960 [14]	B141.5 × 13.92	4382.3	0.82	973.2	2193.4	1.822	0.317	1981.1	0.903
H1-960 [14] ^a	$H211.1\times209.8\times13.96\times13.93$	1882.5	18.67	973.2	4682.7	0.813	0.567	4300.3	0.918
H2-960 [14]	$H209.5\times210.8\times13.93\times13.93$	2883.7	4.92	973.2	4282.2	1.238	0.519	4147.1	0.968
H3-960 [14]	$H209.9 \times 211.0 \times 13.92 \times 13.87$	4381.5	4.83	973.2	2322.8	1.879	0.282	2157.9	0.929
H-3-80-1 [15]	$H171.25 \times 154.50 \times 20.99 \times 11.52$	3320.0	2.08	540.9	1913.0	1.365	0.430	2034.2	1.063
H-3-80-2 [15]	H171.25 × 154.70 × 20.98 × 11.36	3304.0	1.70	540.9	2107.5	1.354	0.475	2093.4	0.993
H-5-55-1 [15]	$H245.75 \times 227.75 \times 21.33 \times 11.54$	3320.0	0.33	464.0	4357.5	0.858	0.763	4288.0	0.984
H-5-55-2 [15]	$H245.50 \times 229.00 \times 21.15 \times 11.62$	3320.0	3.13	502.5	4290.0	0.889	0.695	4345.7	1.013
H-/-40-1[15]	$H_{317,25} \times 308.75 \times 21.03 \times 11.47$	3320.0	3.00	540.9	7590.5	0.682	0.857	/381./	1.021
П-7-40-2 [15] Р 9 90 1 [16]	$P_{1102} \times 1140$	2220.0	2.00	505.8	11225	1 200	0.845	11/1.5	1.051
B-8-80-7 [16]	$B110.5 \times 11.40$ B112.0 $\times 11.40$	3260.0	0.60	505.8	1/73 5	1.200	0.492	1/03.2	0.052
B-12-55-1 [16]	$B1565 \times 11.43$	3260.0	4 90	505.8	2591.0	0.866	0.051	2375.1	0.552
B-12-55-2 [16]	$B156.3 \times 11.42$	3260.0	3.80	505.8	2436.5	0.867	0.728	2418.6	0.993
B-18-38-1 [16]	$B220.2 \times 11.46$	3260.0	2.40	505.8	3774.0	0.602	0.780	4146.6	1.099
B-18-38-2 [16]	B220.8 × 11.46	3260.0	3.40	505.8	4010.0	0.601	0.826	4120.5	1.028
L1-H10 [17]	$H225.2 \times 151.6 \times 10.82 \times 10.82$	2120.0	2.12	550.2	1622.5	1.029	0.538	1640.9	1.011
L2-H10 [17]	$H222.3 \times 151.8 \times 10.39 \times 10.39$	2719.0	2.72	550.2	1141.5	1.315	0.395	1211.2	1.061
L3-H10 [17]	$H221.3 \times 151.8 \times 11.08 \times 11.08$	3318.0	3.32	550.2	839.5	1.600	0.274	981.1	1.169
L1-H10 [17]	$\text{H226.7}\times149.9\times12.74\times12.74$	2120.0	2.12	515.7	2128.0	1.006	0.646	1814.6	0.853
L2-H10 [17]	$H225.2\times150.8\times12.47\times12.47$	2720.0	2.72	515.7	1298.0	1.281	0.402	1363.7	1.051
L3-H10 [17]	$H227.5\times151.6\times12.65\times12.65$	3321.0	3.32	515.7	1143.0	1.557	0.347	1059.8	0.927
B-30-1 [18]	$B236.23 \times 16.20$	2811.0	27.80	624.0	5771.5	0.514	0.649	6447.5	1.117
B-30-2 [18]	B236.47 × 16.10	2812.0	4.90	772.0	9751.5	0.571	0.890	9643.7	0.989
B-50-1 [18]	$B192.37 \times 16.02$	3610.0	0.90	772.0	6444.5	0.914	0.739	7299.6	1.133
B-50-2 [18]	$B192.52 \times 16.02$	3612.0	2.30	772.0	/180.0	0.914	0.822	6934.0	0.966
B-/U-1[18]	$B140.88 \times 10.07$	3610.0	0.10	//2.0	3258.5	1,286	0.526	3/0/.3	1,138
D-/U-2[18]	D140.48 × 10.08	3009.0	1.50	772.0	2897.0	1.290	0.469	3444.8 9496.0	1.189
H_30_2 [18]	H260 35 × 260 82 × 16 25 × 16 25	2011.0	2.00	772.0	800/ 0	0.585	0.914	0400.9 8891 5	0.999
H-50-2 [10]	$H200.33 \times 200.02 \times 10.23 \times 10.23$ H236 30 × 241 75 × 16.02 × 16.02	2010.0	0.50	772.0	7207.0	0.565	0.537	7663.6	1 063
H-50-2 [18]	$H238.15 \times 240.47 \times 16.16 \times 16.16$	2911.0	1.00	772.0	7124 5	0.916	0.832	7456 5	1.005
H-70-1 [18]	$H204.78 \times 209.21 \times 16.26 \times 16.26$	3511.0	2.80	772.0	3039.0	1.263	0.410	4021.3	1.323
H-70-2 [18]	$H205.24 \times 209.38 \times 16.24 \times 16.24$	3512.0	1.50	772.0	3690.0	1.262	0.498	4248.5	1.151
B-120-45 [22] ^a	$B120.68 \times 12.54$	3392.0	45.52	563.0	861.9	1.261	0.282	898.4	1.042

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