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Numerical simulation of kink zone instability in fractured rock masses

J.-F. Noël*, G. Archambault

Université du Québec à Chicoutimi, 555 Boul. de l'Université, Saguenay, Québec, Canada, G7H 2B1

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ABSTRACT

This paper investigates the conditions favourable to the development of a kink zone instability in jointed rock masses in relation with joint properties such as friction angle, frequency of occurrence, and orientation. Numerical simulations using the distinct element code UDEC shows that kink zone instabilities develop when primary joints are oriented from 5° to 30° relatively to the major principal stress under confining pressures below 5.0 MPa. Also, results allow the definition of a failure criterion for kink zone instabilities based on geometrical properties related to the dilatancy rate. The failure criterion is $\tau_{KZI} = \sigma_n \tan \psi$ where ψ is defined as the rotational dilatancy angle.

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1. Introduction

The stability of rock slopes, underground openings and other excavations in intensely foliated or in jointed rock masses, may be at risk with an unusual failure mode by kink zone instability (KZI). This deformation structure is a tabular zone, transverse to joints, bounded by sub-parallel surfaces called kink planes (KP). The “internal” layers (those between the KPs) are sharply deflected and rotated with respect to the “external” ones. This type of instability is developed by buckling instability in brittle, ductile foliated or layered rock. Jointed and fractured rock masses also show the development of KZI as a mode of failure or deformation. Goodman and Kieffer [1] classified this mode of failure as “buckling” and “kink band slumping”. Geomechanical designs of excavations in jointed and foliated rock masses rarely take into account such a mode of failure, and very few studies considered this problem.

KZI has been observed within biaxial tests on rectangular block models composed of square cross-section rods, piled up in a staggered joint pattern (Fig. 1) [2–5]. While some important parameters were tested (joints orientation and frequency; confining pressure), paucity of the data lead to incomplete interpretation of the phenomenon. Recently, numerical modelling, using the 2D distinct element code UDEC, has proven to be suitable to simulate the formation of KZI within rock slopes [6,7] and to reproduce numerically physical biaxial tests [8,9]. Various joint parameters (orientation, surface friction, stiffness) and

boundary conditions (confining pressure, boundary friction) were tested on the numerical biaxial model [8], but the high deformation rates used simulated more a dynamic loading than a static one. Optimal boundary conditions to simulate KZI in a numerical model under static biaxial loading were determined [9], but no new data on the governing parameters were given.

The present paper considers the effects of the governing parameters (joint frequency, confining pressure, primary joints surface friction, and primary joints orientation) on the jointed rock mass strength and on the KZI's geometry. New results come from numerical simulations using UDEC software, and are compared with physical biaxial tests from the literature [2]. Representation of failure envelopes in the Mohr τ – σ_n space leads to the definition of a failure criterion based only on dilatancy.

2. Basic concepts of KZI

Kinking is the result of buckling instability and unstable yielding on planes of anisotropy. The main mechanisms of this rotational deformation are passive slip along the planes and simultaneous rotation of these planes. Phenomenological models, geometrical properties, and deformation characterization are developed in the geology literature on kink banding in strongly foliated rock bodies [10–17]. KZIs are characterized by the angular relationships between α , β , ω , θ_{KP} and the width (W) (Fig. 2).

2.1. Geometry and formation

Two general models have been proposed to explain the formation of kink bands in highly foliated geological material:

* Corresponding author. Tel.: +1 418 545 5011x5646; fax: +1 418 545 5012.

E-mail addresses: jean-francois.noel1@uqac.ca, jfnoel@uqac.ca (J.-F. Noël).

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