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# Physically based coefficient for streamflow estimation in ice-covered channels

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#### ABSTRACT

Measuremental and methodological problems arising from the presence of ice cover on waterways have seriously hindered the progress in predicting timely and accurate streamflow, which is a prerequisite for the effective management of ice-covered waterways (e.g. streams, rivers, channels, and canals). Among various methods in the attempt to address this issue, the most attractive is the physically-based method, which does not require abundant, and often unobtainable, data. This paper develops a physically based *K*-factor method, in the form of a generalized Manning equation, for discharge estimations in ice-covered channels. The main advantage of this method is that it enables the prediction of streamflow in ice-affected seasons by a *K*-factor adjustment of the stage-discharge relationship developed in the open-water seasons. The results of parameter estimation show that the *K*-factor is only related to the boundary resistance ratio, which is parameterized by two exponents of the two-power law for describing the vertical streamwise velocity profile. Experimental measurements collected from the literature were used to test the applicability of the proposed formula, and to compare its relative accuracy with the Sabaneev and Larsen formulae. The comparative results indicate that the proposed method is more accurate in estimating streamflow of ice-covered channels than these two widely-used formulae based on traditional two-layer hypothesis.

#### 1. Introduction

In cold regions, timely and reliable estimates of streamflow throughout the year, especially in the ice-affected season, are essential for supporting the appropriate management of ice-infested waterways (e.g. streams, rivers, channels, and canals). This includes the control of ice-related floods (Ambtman and Hicks, 2012), design and operation of water transfer canals (Altunin et al., 1981; Guo et al., 2017), assessment of river water quality (Healy and Hicks, 2004), and aquatic habitat maintenance (Lind et al., 2014). During ice-free periods, discharge estimates are usually made by continuously monitoring water levels at gauging stations and then converting these data to discharge using established stage-discharge rating curves (Morse and Hicks, 2005; Chokmani et al., 2008). However, when channels are covered with ice in winter, flow under ice is retarded by an additional boundary, which halves hydraulic radius, increases flow resistance, and thence reduces channel conveyance capacity (Davar and Elhadi, 1981; Chokmani et al., 2008; Ghareh Aghaji Zare et al., 2016). Consequently, the rating curve derived for the open-water periods is not applicable during ice-affected

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periods (Teal et al., 1994; Chen et al., 2016). Moreover, because of spatial and temporal variations in ice roughness, it may be impossible to establish a unique rating curve for the ice-affected season (Morse and Hicks, 2005). The determination of ice-affected streamflow entails direct measurements using the velocity-area method (Teal et al., 1994; Attar and Li, 2012) or alternative methods, such as the one-point method, two-point method, and complete-profile method (Walker, 1991). Operationally, direct measurements are labor-intensive and place hydrometric agencies at personal risk (Healy and Hicks, 2004). The development of an analytical method capable of simply and accurately estimating discharge during the ice-affected period is urgently needed.

Since the 1920s, numerous studies have been devoted to ice-affected discharge assessment of ice-covered channels. Notable work has been periodically reviewed by Uzuner (1975), Walker (1991), Chokmani et al. (2008), and Ghareh Aghaji Zare et al. (2016). The methods reviewed could be mainly classified into hydrologic, hydraulic, statistical, and artificial intelligence. The hydrologic method (including the hydrographic method), summarized by Walker (1991), such as base-flow



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Nomenclature		S 11	channel sidewall lateral slope (dimensionless)
The following symbols are used in this paper:		u U <sub>max</sub>	maximum streamwise velocity (m/s)
,		V	cross-sectional mean velocity (m/s)
Α	cross section area (m <sup>2</sup> )	$Y_1$	elevation of the maximum velocity (m)
В	width of a rectangular channel (m)	α, β	regression coefficients (dimensionless)
$B_1, B_2$	widths of channel bottom and the underside of ice cover in	δ	aspect ratio (dimensionless)
	a trapezoidal channel, respectively (m)	$\delta_*$	critical aspect ratio for distinguishing wide channel form
Ε	relative error (dimensionless)		narrow channel (dimensionless)
$E_Q$	relative error of flow discharge (dimensionless)	ε <sub>V</sub> , ε <sub>n</sub>	acceptable iteration accuracies for the bulk velocity and
$f_1, f_2$	Darcy-Weisbach resistance coefficients (dimensionless)		the Manning coefficient, respectively
g	gravitational acceleration (m/s <sup>2</sup> )	η	constant related to flow rate in the two-power law (m/s)
Η	total flow depth (m)	θ	angle of side-wall with horizontal direction (rad)
$h_1$	elevation of zero shear stress (m)	κ	von Karman constant (dimensionless)
$K, K_L, K_S$	K-factors in the proposed formula, the Larsen and	$\lambda_1, \lambda_2$	correction factors (dimensionless)
	Sabaneev formulae, respectively (dimensionless)	ρ	mass density of water (kg/m <sup>3</sup> )
Μ	number of the test runs (dimensionless)	τ	boundary shear stresses acting on the fixed boundaries (N/ $$
$m_1, m_2$	exponents in the two-power law (dimensionless)		m <sup>2</sup> )
n	Manning resistance coefficient (s/m <sup>1/3</sup> )	$\tau_{zx}$	shear stresses along the x-axis in the z-x plane (N/m <sup>2</sup> )
$n_{0S}, n_{0L}$	composite Manning coefficients estimated by the	$\psi_1, \psi_2$	constants (dimensionless).
_	Sabaneev and Larsen formulae, respectively (s/m <sup>1/3</sup> )	<b>0</b> 1 · · ·	
Р	wetted perimeter (m)	Subscripts	
Q	flow discharge (m <sup>°</sup> /s)		and a state of the
R	hydraulic radius (m)	L	subscript denoting parameter for arbitrary subsection
R <sub>e</sub>	Reynolds number (dimensionless)	0	numbered l
$r_m$	ratio of the two exponents in two-power law (dimension-	0	subscript denoting composite values for the entire flow
*	ratio of the Manning coefficients in two flow layers (di		section.
I <sub>n</sub>	mensionless)	Superscripts	
$r_{\gamma}, r_{h}$	flow depth ratios for the traditional and modified two-		
	layer hypothesis, respectively (dimensionless)	k	iteration times.
$S_0$	energy slope (dimensionless)		

recession, K-factor determination, interpolated discharge, and backwater shift, is generally unsuitable for timely estimates of winter discharge because of its irreproducibility, excessive time-consumption, and relatively low precision of predictions (Hamilton et al., 2000; Chokmani et al., 2008). The statistical method uses, for example, conductance correlation and multiple regression to establish an empirical relationship for discharge prediction (Walker, 1991). The artificial intelligence technique, such as artificial neural networks, is an alternative predictor for winter discharge using historical data (Chokmani et al., 2008). The statistical and artificial intelligence methods are data-driven procedures that require abundant historical meteorological and hydrometric data. Hence, these two methods are difficult to apply to discharge predictions for ungauged streams and rivers, and for designs of planed channels and canals. The limitations of poor in-situ databases on method selection and the requirements of reliable discharge estimates for river management result in the widespread use of the hydraulic method in estimating winter discharge under ice-affected conditions.

Of the hydraulic methods available for calculating winter discharge, the Manning equation has advantages because it does not require flow velocities, which are difficult to obtain during ice-affected periods. Icecovered channels are typically composite channels with varying degrees of roughness along their wetted perimeters (Sukhodolov et al., 1999). Pioneering work on ice-covered channel hydraulics was thus mainly centered on the derivation of the composite roughness coefficient that incorporates the frictional effect of fixed boundaries (Nezhikhovskiy, 1964; Larsen, 1973; Uzuner, 1975). Various formulae have been suggested for estimating the composite roughness for ice-covered channels with parabolic sections (Larsen, 1969; Uzuner, 1975; Ghareh Aghaji Zare et al., 2016), but few have considered the rectangular or trapezoidal sections that are widely used in the design of water delivery channels (Chow, 1959). Moreover, if using these existing methods, we need to develop separate stage-discharge relationships for ice-free and ice-affected periods in cold region rivers. Given the easiness in collecting data during ice-free periods, there is a potential alternative method to streamflow estimation during ice-affected periods by using the well-established stage-discharge relationships developed during icefree periods. The objective of this paper was, therefore, to develop a physically-based formula for estimating the discharge capacity during ice-free and ice-affected periods.

The paper is organized as follows. A new analytical predictor is derived for streamflow estimation in ice-covered channels in Section 2. Section 3 discusses the methods for estimating the parameters in the proposed formula. In Section 4.1, field observations and laboratory measurements are documented to verify performance of the proposed formula. Section 4.2 compares the relative accuracy with two other frequently-used formulae, discusses the delimitations of the two-layer flow, and pinpoints the scope of applicability of the proposed formula. Finally, the main conclusions are presented in Section 5.

#### 2. Theoretical analysis

Although the presence of ice cover alters the structure of the underlying flow, quasi-uniform conditions also prevail in cold region waterways during winter (Beltaos, 2011). Therefore, the flow velocity may be related to the resistance coefficient using a conventional friction formula, e.g. the Manning equation (Chow, 1959; Yen, 1992):

$$V_0 = \frac{1}{n_0} R_0^{2/3} S_0^{1/2} \tag{1}$$

where V is the sectional average velocity, R is the hydraulic radius, n is the Manning resistance coefficient, S is the energy slope, and the

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